

Chapter 7

BMP UPGRADES AND RETROFITS

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7.0. INTRODUCTION

Ideally, as land is developed structural controls are implemented to control stormwater runoff impacts. However, controlling stormwater from new development and redevelopment alone will not solve problems resulting from earlier development that did not incorporate stormwater management. Retrofitting is the process by which various kinds of controls are applied to reduce the water quantity and quality impacts from *existing* developed areas. The USEPA is requiring retrofitting as part of the MS4 permitting process, but non-MS4 communities also have runoff problems issuing from older developed areas.

Stormwater retrofits help restore watersheds by providing stormwater treatment in locations where practices previously did not exist or were ineffective. They are typically installed within the stream corridor or upland areas to capture and treat stormwater runoff before it is delivered to receiving waters. Retrofits are the primary practice used to restore small watersheds since they can remove pollutants, promote more natural hydrology and minimize stream channel erosion and minor flooding.

Due to the fact that they are intended to serve existing problem areas, retrofits are typically the responsibility of the local government, which must mitigate property flooding, reduce streambank erosion, or comply with TMDL or other water quality regulatory requirements. Localities can also negotiate some private sector retrofit projects through compliance offset options in the local SWM program (e.g., using fee-in-lieu funds for retrofit projects, allowing off-site compliance where total compliance can't be achieved on the development site, etc.).

Retrofits must be integrated with existing and often diverse urban development, and they assume a wider range of forms than structural controls installed during new development. Space constraints, construction costs, acquisition of easements, safety precautions, economic vitality, and property rights all compete with the need to reduce nutrient loadings in the urban environment.

This chapter describes opportunities and techniques for retrofitting existing, developed sites to improve or enhance water quality mitigation functions. This chapter also identifies the conditions for which stormwater retrofits are appropriate, as well as the potential benefits and effectiveness of stormwater retrofits.

Why Retrofitting is Different

Most retrofit designers have some prior experience designing new stormwater practices. It is important, however, to note the many ways that retrofit design differs from the design of new stormwater treatment practices (**Table 7.1** below).

Retrofitting requires a different way of thinking; it requires sleuthing skills to determine what can work at highly constrained sites. Designers need to simultaneously envision restoration possibilities and anticipate potential problems. They must be extremely creative to find and design effective stormwater solutions within the built environment that produce desired watershed-scale results.

Table 7.1. Why Retrofitting is Different from New Stormwater Design

| Urban Retrofit Practices | New Stormwater Practices |
|---|--|
| Construction costs typically 1.5 - 4 times greater | Designers seek the least costly options |
| Requires significant data collection | Much of the data may be borrowed from past designs |
| Assessment and design costs are higher | Focus is on low cost design and construction |
| Sized to meet small watershed restoration objectives (or the best one can do) | Sized to meet local stormwater design standards |
| Typically installed on public land | Installed at new development projects |
| Urban soils often cannot support infiltration | Soils may support infiltration |
| Fingerprinted around existing development and infrastructure | More flexibility on where to locate practices on the site |
| Must be acceptable to adjacent neighbors and landowners | Aesthetics are not always a major design factor |
| Most are publicly maintained and the public expects that they will be | Most require private maintenance, which is often not performed |
| Not all candidate sites are feasible | Nearly all sites are made to work |
| Often tied into the existing stormwater conveyance system | Usually creates the new stormwater conveyance system |
| Integrated with other restoration practices | Stand-alone practices |
| Public investment in watershed infrastructure | Private investment in stormwater infrastructure |
| A site visit is prerequisite for design | Design may occur without site visits |

Source: CWP, 2007

The design, permitting and construction of retrofits are almost always more complex, expensive and time consuming than new stormwater practices. Also, since most projects are sponsored by the public sector, they must meet high standards for performance, community benefit and appearance. Designers should seek to maximize restoration objectives and not merely design toward a rule. The ethical bar for retrofit design is also higher – designers must ensure that their proposed retrofit adds to watershed function and does not impair existing wetlands, streams and forests. The goal is not just to get approval for a development project or secure a stormwater permit, but rather to create a project that will look good, perform well for many decades, and have a reasonable maintenance burden.

7.1. OBJECTIVES AND BENEFITS OF STORMWATER RETROFITS

The objective of stormwater retrofitting is to remedy problems associated with, and improve water quality mitigation functions of, older, poorly designed or poorly maintained stormwater management systems. In Virginia prior to the 1970s, site drainage design did not require stormwater detention for controlling post-development peak flows. As a result, drainage, flooding, and erosion problems are common in many older developed areas of the state. Furthermore, a majority of the stormwater detention facilities throughout the state have been designed to control peak flows, without regard for water quality mitigation. Therefore, many existing stormwater detention basins provide only minimal water quality benefits.

Incorporating stormwater retrofits into existing developed sites or into redevelopment projects can reduce the adverse impacts of uncontrolled stormwater runoff. This can be accomplished through reduction in unnecessary impervious cover, incorporation of small-scale runoff volume reduction practices, and construction of new or improved structural stormwater treatment practices. One of the primary benefits of stormwater retrofits is the opportunity to combine stormwater quantity and quality controls. Stormwater retrofits can also remedy local nuisance conditions and maintenance problems in older areas, and improve the appearance of existing facilities through landscape amenities and additional vegetation.

The retrofit process begins with a diagnosis of how small watershed development is currently degrading stream quality. The reader may consult **Appendix 5-A** of Chapter 5 this Handbook for an extended discussion of the Impervious Cover Model and how it can be used to diagnose the severity of problems in a subwatershed and determine restoration potential.

Setting restoration objectives early in the retrofitting process is extremely important. Restoration objectives define the purpose of retrofitting and target the specific small watershed problems to be solved. A good set of restoration objectives helps identify what pollutants need to be treated, how much storage is needed and where the most cost-effective locations are in the small watershed. Communities around the country have chosen many different restoration objectives to guide their retrofitting efforts. The most common of those objectives are described below.

Fix Past Mistakes & Maintenance Problems. Traditionally, communities have used retrofits to improve their existing stormwater infrastructure (e.g., to fix drainage problems, deal with under-sized culverts, protect water and sewer lines threatened by erosion, or address chronic maintenance problems within individual stormwater practices). These infrastructure retrofits are localized to address a specific problem and are seldom done on a watershed-wide basis. The type of storage usually is tailored to solve the specific problem at the site.

Solve Chronic Flooding Problems. Another common retrofit objective is to solve flooding problems at vulnerable locations within a small watershed. This retrofitting approach focuses on specific stream reaches or flood prone areas. Upstream storage retrofits may be investigated to reduce flood damage in small watersheds that were developed prior to local stormwater or floodplain management requirements. These large retrofits are typically sized to provide storage for extreme flood events (e.g., 25 to 100 year peak discharge control).

Stormwater Demonstration and Education. Many communities embark on retrofitting to demonstrate new stormwater practices on public lands or promote stormwater education and stewardship. As a result, demonstration retrofits are designed for individual site needs rather than to meet watershed goals. Most demonstration retrofits are sized to the treatment volume and intended to introduce new stormwater technologies. Well-designed and highly visible demonstration retrofits are a good tactic to build community support to finance more widespread retrofitting efforts in the future.

Trap Trash and Floatables. The objective of these retrofits is combine pollution prevention, storage retrofits and improved catch basins to trap trash and floatables before they enter receiving waters. Since trash is fairly easy to trap, most retrofits are sized based on a fraction of the treatment volume, although they typically require intensive maintenance after every major

storm event. Retrofit programs to reduce trash have been conducted in diverse locations, including New York City, Los Angeles, Baltimore, Albuquerque, and the District of Columbia.

Reduced Runoff Volumes to Combined Sewers. In recent years, communities have recognized that on-site retrofits can greatly reduce stormwater inputs to combined sewers, thereby reducing the frequency and size of sewage overflows in urban subwatersheds. This retrofit strategy can greatly reduce the size and cost of traditional combined sewer overflow (CSO) abatement systems such as deep tunnels or storage pipes. In many cases, on-site retrofits only need to reduce a fraction of the treatment volume to become a cost-effective technique to reduce CSOs. Rooftop treatment or disconnection is the most common approach to reduce runoff volumes, and they have been applied in diverse settings, including Philadelphia, Pittsburgh, Portland, Milwaukee, and the District of Columbia.

Renovate the Stream Corridor. This objective focuses on installing retrofits to improve the habitat, diversity and overall quality of a stream corridor, whether it is a greenway, stream valley park, or a chain of wetlands or lakes. The retrofits are located in or near the stream corridor and are intended to improve water quality, create wetland and wildlife habitat, daylight urban streams, naturalize the stream corridor or demonstrate creative stormwater practices. Some progressive communities that have utilized retrofits to renovate stream corridors include Staten Island (the Staten Island Bluebelt), Minneapolis (Minnehaha Creek), and the Detroit Metro Area (Rouge River).

Reduce Pollutants of Concern. Pollutant reduction is often a primary objective of local retrofit programs. The reduction may be driven by a TMDL, a local watershed restoration plan or regional directive to reduce pollutant loads. The pollutant of concern may include sediment, nutrients, bacteria, metals and toxins. Retrofits are then systematically applied across a small watershed to achieve a pre-designated pollutant reduction goal. Retrofits are typically sized based on a target treatment volume, although individual retrofits may be under- or over-sized. Examples of communities that have retrofit small watersheds to maximize pollutant removal include Staten Island, various communities in Maryland and North Carolina; Austin, Texas; Santa Monica, California; and Burlington, Vermont.

Systematically Reduce Downstream Channel Erosion. A few communities have sought to reduce downstream channel erosion by installing retrofits in urbanizing watersheds. This approach requires systematic installation of channel protection storage retrofits throughout the stream corridor. The strategy works best in impacted watersheds where the greater storage volume needed for channel protection can be more easily found. In recent years, this restoration objective has been linked to reducing nutrient loads derived from eroding streambanks. Two notable small watersheds where channel protection has been a primary restoration objective are Watts Branch and Minebank Run in Maryland.

Support Stream Restoration. This objective uses upstream retrofits to provide hydrologic control to support downstream restoration projects. Individual retrofits are installed above specific stream reaches where stream restoration is planned. The retrofits may provide recharge, water quality, channel protection, or some combination, depending on the specific design needs of the downstream project. The retrofits regulate the volume, duration, frequency, or peak discharge of

storm flow, thereby creating a more stable and predictable hydrologic regime for the new stream. The long-term success of many stream repair/restoration projects is often depends on effective upstream retrofits. Notable examples of paired retrofit/stream repair projects on individual streams include Accotink Creek in Virginia and Watts Branch, Longwell Branch and Wheaton Branch in Maryland.

Comprehensive Watershed Restoration. The ideal objective is a comprehensive approach to restore small watersheds that integrates retrofits in the context of other goals such as stream repair, riparian reforestation, discharge prevention, upland reforestation, pollution source control and improved municipal practices.

7.2. WHEN IS RETROFITTING APPROPRIATE?

Site constraints commonly encountered in existing, developed areas can limit the type of stormwater retrofits that are possible for a site as well as their overall effectiveness. Retrofit of an existing stormwater management facility, consistent with the design specifications contained in the Virginia Stormwater BMP Clearinghouse web site, may not be possible due to site-specific factors such as the location of existing utilities, buildings, wetlands, maintenance access, and adjacent land uses. **Table 7.2** below lists site-specific factors to consider in determining the appropriateness of stormwater retrofits for a particular site.

Table 7.2. Site Considerations for Determining the Appropriateness of Stormwater Retrofits

| Factor | Consideration |
|--|---|
| Retrofit Purpose | What are the primary and secondary (if any) purposes of the retrofit project? Are the retrofits designed primarily for stormwater quantity control, quality control, or both? |
| Construction/Maintenance Access | Does the site have adequate construction and maintenance access and sufficient construction staging area? Are maintenance responsibilities for the retrofits clearly defined? |
| Subsurface Conditions | Are the subsurface conditions at the site (soil permeability, depth to groundwater/bedrock, presence of karst geology, etc.) consistent with the proposed retrofit regarding subsurface infiltration capacity and constructability? |
| Utilities | Do locations of existing utilities present conflicts with the proposed retrofits or require relocation or design modifications? |
| Conflicting Land Uses | Are the retrofits compatible with the land uses of adjacent properties? |
| Wetlands, Sensitive Water Bodies, Karst Topography, and Vegetation | How do the retrofits affect adjacent or down-gradient wetlands, sensitive receiving waters, karst features, and vegetation? Do the retrofits minimize or mitigate impacts where possible? |
| Complementary Restoration Projects | Are there opportunities to combine stormwater retrofits with complementary projects such as stream stabilization, habitat restoration, or wetland restoration/mitigation? |
| Permits and Approvals | Which local, state, and federal regulatory agencies have jurisdiction over the proposed retrofit project, and can regulatory approvals be obtained for the retrofits? |
| Public Safety | Does the retrofit increase or reduce the risk to public health and safety? |
| Cost | What are the capital and long-term maintenance costs associated with the stormwater retrofits? Are the retrofits cost-effective in terms of anticipated benefits? |

Source: CT DEP, 2004

Newly designed and installed stormwater management facilities are typically more effective than retrofitted facilities in reducing pollutant loads. However, in most cases, some improvements in stormwater quantity and quality control are possible with retrofits, especially if a new use is planned for an existing development or an existing storm drainage system is upgraded or expanded. Incorporation of a number of runoff volume reduction practices or a treatment train approach may be necessary to achieve the desired level of effectiveness. It should also be recognized that increased stormwater quantity and peak flows often causes channel erosion, resulting in some of the most severe impacts to receiving waters and wetlands (Claytor, Center for Watershed Protection, 2000). Therefore, stormwater quantity control functions provided by existing stormwater management facilities should not be compromised significantly in exchange for pollutant removal effectiveness.

7.3. EXAMPLES OF SUCCESSFUL STORMWATER RETROFIT PROJECTS

Built-out spaces often require innovative ways to treat stormwater – sometimes because runoff and water quality problems have increased along with development, and sometimes because stormwater requirements were less stringent when the original development took place. When it comes to urban stormwater retrofitting, every little bit counts. But finding the space and means to incorporate stormwater measures is often a challenge. The following examples demonstrate how three sites across the nation have managed it.

7.3.1. Liberty Centre Parking Garage

In the case of the Liberty Centre Parking Garage in Portland, Oregon, two planters were squeezed between two exterior walls of the parking garage and the sidewalks (**Figure 7.1**). The planting areas make up just 5 percent of the drainage catchment area of the 36,000 square foot parking deck, but they can infiltrate almost all of the stormwater from a 2-year rainfall event. The project's reduction in stormwater volume and its use of native vegetation have qualified it for LEED (Leadership in Energy and Environmental Design) certification.



Figure 7.1. Planting Areas at the Liberty Centre Parking Garage

Source: Aird, 2009 (Ashworth Pacific, Inc.)

The project's goals were to (1) reduce the volume of stormwater flowing into the combined sewers in the area and (2) to improve the quality of water flowing into the nearby Willamette River. The planters absorb and infiltrate at least 2 inches of stormwater per hour. More details regarding the project's design and costs can be found in *Aird (2009)*.

7.3.2. Burnsville Rain Gardens

Stormwater used to flow down Rushmore Drive in Burnsville, Minnesota, right into Crystal Lake. The amount of phosphorus it carried was causing algae blooms in the lake, which affected recreation. Today, the suburban street is a site of a demonstration project where 17 of 25 homes have rain gardens (**Figure 7.2**) that capture and infiltrate runoff before it reaches the lake.



Figure 7.2. Front Yard Rain Garden, Burnsville, MN

Source: Aird, 2009 (Barr Engineering)

There was not enough space for traditional stormwater ponds in the neighborhood, which was built in the 1980s. However, Rushmore Drive has a gentle topography, sandy soils, and 15-foot rights-of-way (from the edges of the curb) that provide plenty of space for the rain gardens. The City initiated and funded the project, but there was a fairly significant educational component in order to obtain homeowner cooperation. Ultimately, more than 80 percent of the homeowners in the 5.3 acre drainage area wanted to participate, motivated by the opportunity to be part of improving local water resources.

According to city officials, the homeowners appear to be proud and happy with their rain gardens. They view them as amenities to their homes as well as water quality improvement measures, so they take good care of their own rain gardens. The project is different from most rain garden projects, which tend to be more spread out. However, city officials have concluded that the real positive impact is when rain gardens are clustered together.

Five years of monitoring the project indicates that the project has reduced runoff to the lake by 90 percent, when compared to similar neighborhoods nearby without rain gardens. In nearly all cases, the gardens have been able to infiltrate and treat at least 0.9 inch of stormwater runoff. Most gardens have dried within three or four hours, and there haven't been any adverse effects from ice buildup in winter. More details regarding the project's design and costs can be found in *Aird (2009)*.

7.3.3 Broadview Green Grid Project

Contrary to popular belief, Seattle, Washington, doesn't receive an excessive amount of annual rainfall. However, the 36 inches it does receive falls on slopes of dense glacial fill and impermeable urban surfaces. Stormwater there is causing the familiar problems: polluted runoff, eroded stream channels, and impaired wildlife habitat.

In 1999, Seattle Public Utilities (SPU) began its Natural Drainage System program. It focuses on increasing pervious areas along street edges by redesigning existing streets and installing landscaping that infiltrates stormwater efficiently. In 2004, SPU completed its Broadview Green Grid Project in partnership with the Seattle Department of Transportation. The project covers approximately 32 acres, almost an entire sub-basin of Piper's Creek, which leads to the Puget Sound.

The goals of the project were to move stormwater off of roads and properties, slow it down, and allow it to infiltrate into the soil before it reached Piper's Creek. This would recharge the groundwater and sustain the creek during the dry summer months, as well as reduce erosion in the creek and the amount of pollutants – oil, grease, heavy metals, pet waste, sediments, fertilizers, and pesticides – emptying into it.

The project encompasses 15 blocks of residential property, but the entire project is installed on public land: across the width of the streets and easements on both sides, for a total width of about 60 feet. As in Burnsville, city officials surveyed the residents to ensure they would support the project.

The roadway design affected only three north-south streets, which slope down to the west. They began as straight, 25-foot wide roadways with two-way traffic and continuous parking space on both sides. They've been narrowed to about 19-20 feet wide, and they now meander slightly, slowing the runoff and guiding it off the road. Every street still has two-way traffic, one parking lane, and room for emergency vehicles designed for urban areas. Some streets have a sidewalk. The narrow, winding streets provide a bonus for the residents – traffic moves very slowly, discouraging cut-through traffic and providing much more safety for pedestrians.

The easements on both the north-south and the east-west streets have some traditional drainage features, such as culverts and catch basins, as well as swales, bioretention cells or rain gardens, and cascades. The steeper the street, the more grade control was used.

The east-west streets have very steep downhill slopes. The swales, which are along only one side of the streets, are giant “swale cells.” They’re divided by concrete weirs, each with a notch to control the flow of water. The weirs act as a series of steps that slow stormwater as it flows down into the swales. Rock walls line one side of these swales (**Figures 7.3 and 7.4**).



Figure 7.3. Seattle's 107th Street Cascade Before Planting.

Source: Aird, 2009



Figure 7.4. Finished Swale Cells Along Seattle's 107th Street, an East-West Street. Source: Aird, 2009

The north-south streets, which have cross slopes to a maximum of approximately 8 degrees, have 20-foot easements with swales along both sides of the streets. Rock walls line one side of the swales to maximize their area (**Figure 7.5** below). The bioretention cells are on flatter ground and aren't designed to retain the high volumes of stormwater that swales do.

All of the features are landscaped with native plants, whose roots help stabilize the soil, absorb runoff, and remove pollutants. Smaller trees and shrubs were chosen that wouldn't outgrow the easements, as well as grasses, sedges and rushes in dense groups and wetland plants in lower, moister areas. Most of the swales are designed to infiltrate $\frac{1}{2}$ -inch of stormwater per hour and all stormwater within three days. There is never more than 12 inches of standing water while it's raining. Any water that doesn't infiltrate flows into a pool where it's treated and detained before continuing into the downstream stormwater network.



Figure 7.5. Swale and Curves Along Phinney, Avenue, a North-South Street in Seattle. Source: Aird, 2009

The homeowners maintain the landscape. Most of SPU's maintenance costs consist of keeping the landscape mulched. Sedimentation structures, which accumulate pollutants attached to dirt and particles, are cleaned out once a year. According to SPU projections, natural drainage systems such as this are costing at least 25 percent less than traditional stormwater systems because of decreased construction and maintenance costs. They also offer aesthetic improvements that traditional systems do not.

For examples of retrofits using some manufactured stormwater management devices, see Rafter (2008).

7.4. RAINFALL, RUNOFF AND RETROFITS

Once core retrofit objectives are selected, they need to be translated into subwatershed sizing criteria. For this reason, the retrofit team must understand the relationship between rainfall, runoff and retrofits in their community. Retrofitting is fundamentally driven by the distribution of rainfall events. This section introduces the concept of the rainfall frequency spectrum, and how it can be used to define the target runoff volumes for retrofitting.

In the course of a year, many precipitation events occur within a community. Most events are quite small but a few can be several inches deep. A rainfall frequency spectrum describes the average frequency of the depth of rainfall events that occur during a normal year (adjusted for snowfall). **Figure 10.1** (in **Chapter 10** of this Handbook) provides an example of a typical rainfall frequency spectrum that shows the percent of rainfall events that are equal to or less than the indicated rainfall depth. As can be seen, the majority of storms are relatively small but a sharp upward inflection point occurs at about one-inch of rainfall. A rainfall frequency spectrum can outline up to five different zones that define targets for different stormwater treatment objectives, as follows:

Recharge. This targets rainfall events that create little or no runoff but contribute much of the annual groundwater recharge at a site. (NOTE: The Virginia Stormwater Management Law and Regulations do not currently include any independent requirements for recharging groundwater, but local governments may establish their own criteria using the authority in the Law and Regulations to adopt more stringent criteria. See **Appendix 10-A** at the end of **Chapter 10**.)

Water Quality. This targets rainfall events that deliver the majority of the stormwater pollutants during the course of a year (denoted as Treatment Volume, or T_v).

Channel Protection. This targets storms that generate bankfull and sub-bankfull floods that cause stream channel erosion and enlargement.

Overbank Floods. This targets large and infrequent storm events that spill over to the floodplain and cause damage to infrastructure and streamside property.

Extreme Storms. This controls the largest, most infrequent and most catastrophic floods that threaten structures and public safety (e.g., commonly known as the 100-year storm). (NOTE: The Virginia Stormwater Management Law and Regulations do not currently include any independent requirements for extreme flood protection, but local governments may establish their own criteria using the authority in the Law and Regulations to adopt more stringent criteria.)

In general, retrofitting focuses on the lower end of the rainfall frequency spectrum (i.e., managing runoff for recharge, water quality and channel protection). Small watershed retrofitting to control overbank floods or extreme storms is rarely attempted, since it is hard to get enough retrofit storage to manage runoff at this end of the spectrum. As a result, flood mitigation projects are normally installed to prevent problems within a specific flood-prone reach and not on a watershed-wide basis.

Retrofit teams will achieve more precision in their results if they develop localized retrofit sizing criteria based on their own rainfall frequency spectrum analysis, using the following guidance (CWP, 2007):

1. Obtain a long-term rainfall record from the adjacent weather station (daily precipitation is fine, but try to obtain at least 30 years of daily records). NOAA has several websites with long-term rainfall records (see <http://ols.nndc.noaa.gov>).

2. Edit out small rainfall events that are 0.1 inch or less. Also edit out snowfall events that do not immediately melt.
3. Using a spreadsheet or simple statistical package, analyze the rainfall time series and develop a frequency analysis to determine the percentage of rainfall events greater than or equal to a given numerical value (e.g., 0.2, 0.5, 1.0, 1.5 inches, etc.).
4. Construct a curve showing rainfall depth versus frequency, and create a table showing rainfall depth values for 50, 75, 90 and 95% frequencies.
5. Use the data to define the recharge event (20-50%), treatment event (90%), and one-year storm (99%).

If a community is large or has considerable variation in elevation or aspect, the RFSA should be conducted at multiple stations. Other regional and national rainfall analyses, such as TP-40 (NOAA) or USGS, should always be used for rainfall depths or intensity greater than one-year return frequency (e.g., 2, 5, 10, 25, 50 or 100 year design storm recurrence intervals). The rainfall frequency spectrum provides a strong basis to set targets for the desired water quality, runoff reduction or channel protection volume to seek in a subwatershed, as described below.

Setting Treatment Volume Targets for Retrofitting. The water quality treatment retrofit goal is to capture and treat the 90% storm (defined by the state regulations as the 1-inch rainfall event) or a local rainfall frequency spectrum (?). This criterion optimizes runoff capture resulting in high load reduction for many stormwater pollutants. Based on the treatment design storm, it is relatively easy to determine the retrofit storage volume needed at either the site or small watershed scale.

Several practical implications arise when establishing the treatment volume for a small watershed – particularly when it comes to finding enough retrofit sites to meet it. In general, when the T_v is large, fewer retrofit sites can be found that have adequate space to capture and treat it. An optimization point exists between the target volume and expected number of retrofit locations, as shown in **Figure 7.6**.

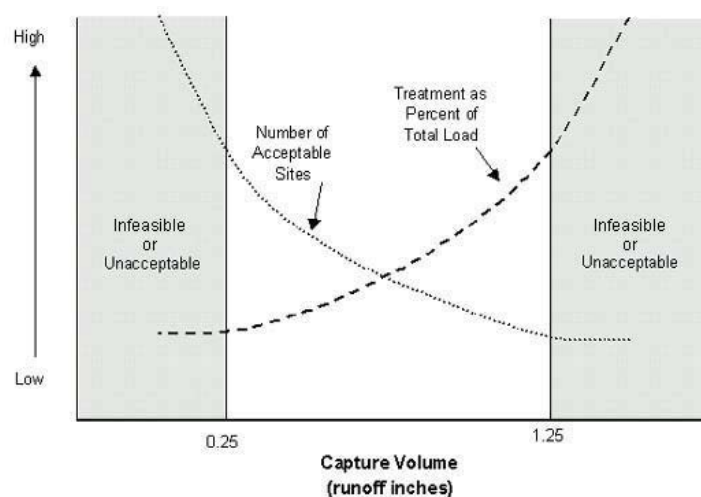


Figure 7.6. Optimization Point for Retrofit Treatment
Source: CWP, 2007

One curve shows how the fraction of subwatershed treatment increases when the capture volume becomes progressively greater. The second curve shows how the number of feasible retrofit sites declines as a function of a higher capture volume. An optimization point exists in most small watersheds where the two curves intersect. The retrofit optimization point also reflects the degree of watershed impervious cover – shifting towards 0.25 inches in highly developed watersheds and as much as 1.25 inches in lightly developed ones. This optimization point is an important factor to define early in the retrofit scoping process.

Setting Runoff Reduction Volume Targets. The target storage volume for runoff reduction typically ranges from 20-50 percent of the T_v and can be attained through canopy interception, rooftop disconnection, infiltration, rainwater harvesting, evaporation or long-term storage. The target runoff reduction volume is determined based on local watershed characteristics, and the desired degree of CSO relief, groundwater recharge or baseflow maintenance. Runoff reduction volumes are deceptively low in comparison to other target volumes. Designers should be aware that most storage retrofits do not reduce much runoff volume, so that dozens or even hundreds of small on-site retrofits may be needed to achieve runoff reduction objectives, depending on the size of the site or watershed. As noted above, the Virginia Stormwater Management Law and Regulations do not currently include any independent requirements for reducing runoff or recharging groundwater, but local governments may establish their own criteria using the authority in the Law and Regulations to adopt more stringent criteria.)

Setting Channel Protection Volume Targets. The recommended channel protection criterion is 24 hours of extended detention for the runoff generated by the 1-year 24-hour design storm. This is generally equivalent to the rainfall depth for the 99% storm. Past practice has resulted in runoff being stored (detention) and gradually released over a 24-hour period so that critical erosive velocities in downstream channels are not exceeded during the entire storm hydrograph. As a very rough rule, the storage capacity needed to provide channel protection is about 60% of the one-year storm runoff volume. However, it is possible that the need for detention facilities can be avoided if sufficient runoff volume reduction is achieved by other control measures. Designers will normally need to define actual channel protection volumes using hydrologic and hydraulic models that simulate specific channel conditions and subwatershed characteristics.

Channel protection storage generally exceeds the treatment volume by 20 to 40%. It may seem intuitive that the channel protection volume should always be higher than the T_v , since the rainfall depth associated with the 99% storm must always be greater than the 90% storm. The key difference is that the T_v is defined as 100% of the runoff volume produced by the 90% rain depth; whereas the channel protection volume is estimated as 60% of the runoff volume produced for the 99% rain depth.

Both the T_v and the channel protection storage volume may be needed to attain certain small watershed retrofit objectives, which effectively doubles the total storage volume needed. The best conditions for finding enough channel protection storage are in small sensitive or impacted watersheds that have a high existing pond density and/or abundant public land in stream corridors. In many cases, complete retrofit channel protection is not possible for the watershed as a whole, but it may be feasible for individual stream reaches where stream repairs are being proposed.

7.5 THE SEARCH FOR WATERSHED STORAGE

Watershed treatment is an important concept when assessing retrofit potential. Designers need to calculate the total water quality treatment volume needed to meet the restoration objectives. The feasibility of capturing and treating this volume will be different in every watershed. Conceptually, subwatershed treatment is represented by the following equation:

$$\text{Total volume} = \text{Storage retrofits} + \text{on-site retrofit storage} + \text{future redevelopment treatment}$$

The redevelopment term reflects future opportunities to provide stormwater treatment within the watershed as land is redeveloped. While redevelopment is not an explicit component of the retrofitting process, it is important to update existing stormwater criteria to take advantage of long-term opportunities to install new/additional treatment measures.

The challenge of retrofitting is to find enough storage to make a real difference in a watershed. The required storage volume can consume a significant percentage of watershed area, particularly when channel protection and flood control storms are being managed.

Retrofitting becomes more and more difficult and costly to pursue as subwatershed imperviousness increases. At lower levels of impervious cover, it is generally possible to find needed storage volumes for water quality treatment and, sometimes, channel protection. Available land to provide water quality and/or channel protection is harder to come by at higher levels of watershed impervious cover (45-60%).

7.6. THE RANGE OF RETROFIT PRACTICES

Retrofits can be classified by the amount of subwatershed area they treat. *Storage retrofits* treat drainage areas ranging from five to 500 acres. By contrast, *on-site* residential retrofits may individually treat as little as 500 square feet of contributing drainage area. On-site, non-residential retrofits normally treat less than five acres of contributing drainage area, and frequently less than one.

Storage and on-site retrofits represent two different approaches to attain treatment storage and involve different design and assessment methods (**Table 7.3**). As a general rule, storage retrofits are the most cost-effective approach to meet most subwatershed restoration objectives, although both retrofit approaches may be needed to get the desired level of subwatershed treatment.

Storage Retrofit Classification. Storage retrofits are classified using common locations in a subwatershed where large storage volumes can be found. The six major storage retrofit locations are described in detail in **Table 7.4**. Most storage retrofits are located on publicly owned or controlled land, and rely on some combination of extended detention, wet pond, constructed wetland or bioretention for stormwater treatment.

On-Site Retrofit Classification. On-site retrofits are classified based on the type or location of impervious area they treat, such as individual rooftops, small parking lots, streets, stormwater hotspots and other small impervious areas. Seven onsite retrofit locations are described in **Table 7.5**. On-site retrofits treat the quality and/or reduce the volume of runoff generated by small

urban source areas and rely on bioretention, filtering, infiltration, swales or rooftop treatment. On-site retrofits are an effective strategy in ultra-urban subwatersheds that lack space for storage retrofits, and can also provide excellent opportunities to improve public awareness and involvement. Most on-site retrofits are normally installed on private land but involve some form of public delivery.

Table 7.3. Two Different Approaches to Retrofitting

| Storage Retrofits | On-Site Retrofits |
|--|---|
| Serve 5 to 500 acres | Serve 0.1 to 5 acres |
| Generally constructed on public land | Generally constructed on private land |
| May need dozens in a subwatershed | May need hundreds in subwatershed |
| Assessed at subwatershed scale | Assessed at catchment/neighborhood scale |
| Moderate cost per impervious acre treated | High cost per impervious acre treated |
| Impractical in ultra urban areas | Practical in ultra-urban areas |
| Permitting can be extensive | Few permits are needed |
| Can address all stormwater control targets | Only provide recharge and water quality control |
| Public construction | Public delivery |
| Use extended detention, wet pond, and wetlands | Rely on bioretention, filtering, infiltration, swales and other treatment practices |

Source: CWP, 2007

Table 7.4. The Six Most Common Storage Retrofit Locations in a Subwatershed

| Where to Look | How to Get Storage |
|--|--|
| Add Storage to Existing Ponds | Add water quality treatment storage to an existing pond that lacks it by excavating new storage on the pond bottom, raising the height of the embankment, modifying riser elevations/dimensions, converting unneeded quantity control storage into water quality treatment storage and/or installing internal design features to improve performance. |
| Storage Above Roadway Culverts | Provide water quality storage immediately upstream of an existing road culvert that crosses a low gradient, non-perennial stream without wetlands. Free storage is created by adding wetland and/or extended detention treatment behind a new embankment just upstream of the existing roadway embankment. |
| New Storage Below Outfalls | Flows are split from an existing storm drain or ditch and are diverted to a stormwater treatment area on public land in the stream corridor. Works best for storm drain outfalls in the 12- to 36- inch diameter range that are located near large open spaces, such as parks, golf courses and floodplains. |
| Storage in Conveyance Systems | Investigate the upper portions of the existing stormwater conveyance system to look for opportunities to improve the performance of existing swales, ditches and non-perennial streams. This can be done either by creating in-line storage cells that filter runoff through swales and wetlands or by splitting flows to off-line treatment areas in the stream corridor. |
| Storage in Road Rights-of-Way | Direct runoff to a depression or excavated stormwater treatment area within the right of way of a road, highway, transport or power line corridor. Prominent examples include highway cloverleaf, median and wide right of way areas. |
| Storage Near Large Parking Lots | Provide stormwater treatment in open spaces near the down-gradient outfall of large parking lots (5 acres plus). |

Source: Adapted from CWP, 2007

Table 7.5. The Seven Most Common On-Site Retrofit Locations in a Subwatershed

| Where | How |
|----------------------------------|---|
| Hotspot Operations | Install filtering or bioretention treatment to remove pollutants from confirmed or severe stormwater hotspots discovered during field investigation. |
| Small Parking Lots | Insert stormwater treatment within or on the margins of small parking lots (less than five acres). In many cases, the parking lot is delineated into a series of smaller on-site treatment units. |
| Individual Streets | Look for opportunities with the street, its right of way, cul-de-sacs and traffic calming devices to treat stormwater runoff before it gets into the street storm drain network. |
| Individual Rooftops | Disconnect, store and treat stormwater runoff generated from residential and commercial rooftops close to the source. |
| Little Retrofits | Convert or disconnect isolated areas of impervious cover and treat runoff in an adjacent pervious area using low tech approaches such as a filter strip. |
| Landscapes and Hardscapes | Reconfigure the plumbing of high visibility urban landscapes, plazas and public spaces to treat stormwater runoff with landscaping and other urban design features. |
| Underground | Provide stormwater treatment in an underground location when no surface land is available for surface treatment. Use this as a last resort at dense ultra-urban sites. |

Source: Adapted from CWP, 2007

7.7. STORMWATER RETROFIT OPTIONS

Stormwater retrofit options include many of the same source control and stormwater treatment practices for new developments that are described in other chapters of this Manual. Common stormwater retrofit applications for existing development and redevelopment projects include:

- Source control retrofit
- Stormwater drainage system retrofits
- Stormwater management facility retrofits
- New stormwater controls at storm drain outfalls
- New stormwater controls at roadway culverts and above roadway crossings
- New stormwater controls for highway rights-of-way
- Individual streets
- Parking lot stormwater retrofits
- In-stream practices in existing drainage channels
- Wetland creation and restoration
- Hotspots
- Individual rooftops
- Little retrofits
- Landscapes/hardscapes
- Underground

7.7.1 Source Control Retrofit

Source control techniques, sometimes referred to as “good housekeeping practices,” attenuate runoff and/or pollutant generation before it enters a storm drain system, (e.g., reducing impervious areas, using pollution prevention practices, covering road salt/sand storage piles, etc.) These are especially important in areas where build-out prevents the establishment of a significant number of new facilities, and where redevelopment will not have a significant impact on water quality.

7.7.2 Stormwater Drainage Systems

Existing drainage systems can be modified to improve water quality mitigation and sediment removal functions. These retrofits alone typically provide limited benefits, but are most successful when used in conjunction with other source controls and stormwater treatment practices. Due to their very nature as an integral part of the stormwater collection and conveyance system and inherent solids trapping function, these retrofits typically have high maintenance requirements. Common examples of stormwater drainage system retrofits include:

Deep Sump Catch Basins with Hoods. Older catch basins without sumps can be replaced with catch basins having four to six-foot deep sumps. Sumps provide storage volume for coarse sediments, provided that accumulated sediment is removed on a regular basis. Hooded outlets, which are covers over the catch basin outlets that extend below the standing water, can also be used to trap litter and other floatable materials. A study conducted in New York City demonstrated that catch basins equipped with hoods increase the capture of floatables by 70 to 80 percent over catch basins without hoods and greatly extend the cleaning interval without degraded capture performance (Pitt, 1999 in NRDC, 1999).

Catch Basin Inserts and Storm Drain Structures. A number of manufactured devices have been developed that can be inserted into storm drains or catch basins to capture sediment and other pollutants directly beneath the grate. These products typically utilize filter media or vortex action for removal of solids from incoming stormwater runoff. These devices are ideally suited for developed sites since they fit inside of or replace existing catch basins, or are installed beneath existing parking lots with minimal or no additional space requirements.

Treatment in the Conveyance System. This retrofit obtains storage within altered zero and first order stream channels that comprise about half of the channel network in most subwatersheds. These channels lack perennial flow, have minimal floodplains and typically have a contributing drainage area of 15 to 50 acres in humid regions. However, these channels rarely show up on local GIS maps (**Figure 7.7** below).

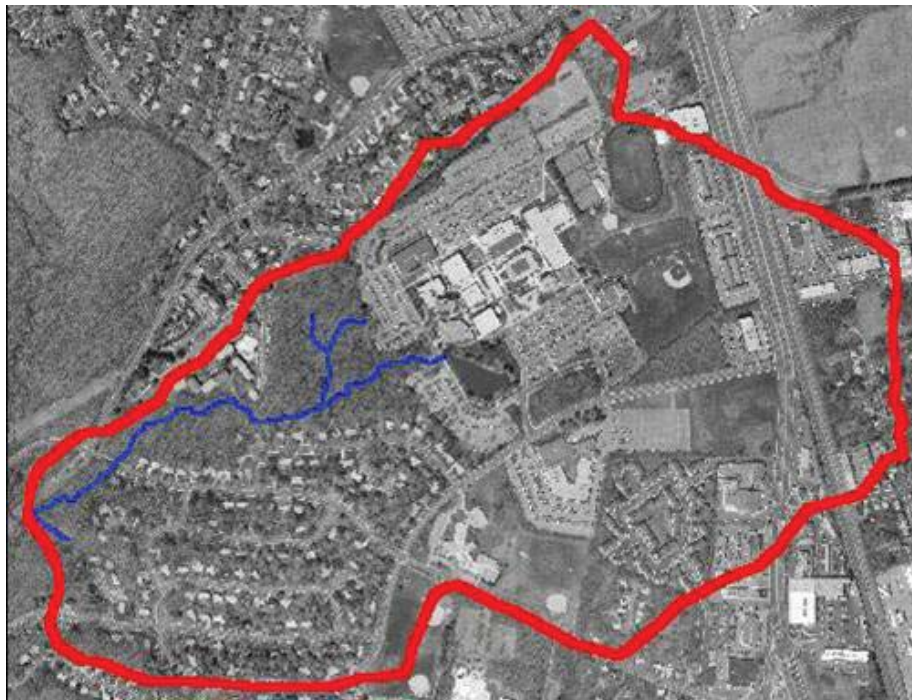


Figure 7.7. Most ditch lines and zero-order streams do not show up on local GIS maps (Source: CWP, 2007)

Conveyance retrofits create storage, bioretention or wetland cells in an existing ditch, swale or non-perennial stream channel (**Figure 7.8** below). Conveyance retrofits are particularly appropriate in small headwater channels that have been channelized and/or hardened in the past.

There are two basic design variants for the conveyance retrofit – *in-channel* designs where stormwater treatment storage is obtained within the channel and *off-channel* designs where the treatment storage is provided in cells adjacent to the channel. In-channel retrofits obtain storage by:

- Installing small weir walls or check dams in the channel to provide more storage
- Converting a channel or ditch into dry swale or wet swale
- Creating a linear series of wetland or bioretention treatment cells in the channel

Off-channel retrofits split storm flows from the channel to an adjacent depression or excavated treatment area (also **Figure 7.8**). Off-channel retrofits can be effective when floodplain reconnection or wetland creation is a subwatershed restoration objective. Constructed wetlands and bioretention are preferred for off-channel applications since they minimize the need for major excavation and embankments.

The stormwater conveyance system is a good location for storage retrofits since the land is usually located in a dedicated easement or right of way.

The ideal conditions for a conveyance retrofit are when the channel has:

- Gradient ranging between 0.5 and 2.0%
- Contributing drainage area of 15 to 30 acres of in humid regions with tight soils. Minimum drainage areas for conveyance retrofits are greater in arid and semi-arid regions with permeable soils
- Been altered to promote efficient drainage (e.g., ditch, swale or concrete-lined channels)
- Less than three feet of elevation difference between the top of bank and the channel bottom
- Been used for roadway drainage in the right of way
- An unutilized parcel of public land located adjacent to the channel.

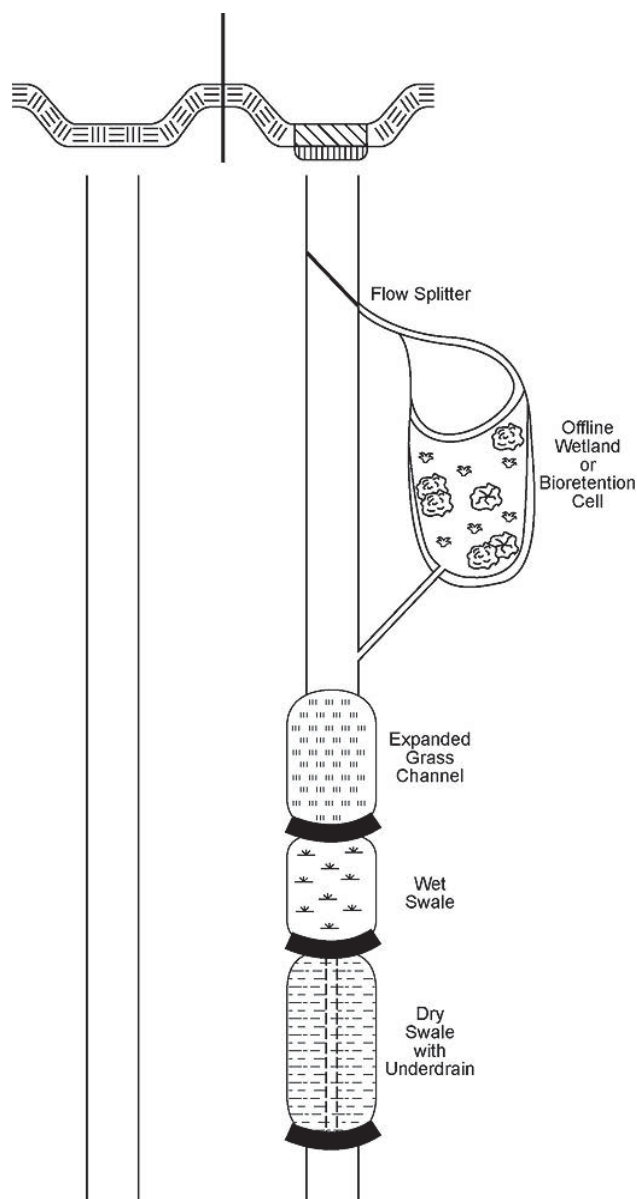


Figure 7.8. Both in-channel or off-channel treatment are possible in a conveyance.

Source: CWP, 2007

Figure 7.9 illustrates several examples of good candidate sites in the conveyance system for retrofit storage.



Figure 7.9. Four opportunities within the conveyance system for retrofitting

Source: CWP, 2007

Conveyance retrofits are generally *not* a good idea when the existing channel:

- Is in natural condition and has adjacent mature forests or wetlands
- Is rapidly degrading/incising or has a knick point advancing upstream
- Has a channel gradient of 5% or more and/or steep side slopes
- Has perennial flow
- Is located close to a residential neighborhood
- Is privately owned or lacks a drainage easement

7.7.3. Stormwater Management Facilities

Stormwater Treatment Options for Retrofitting. Eight different stormwater treatment options can be used for retrofitting. Each treatment option differs greatly in its pollutant removal capability, hydrologic benefit and retrofit suitability. More detailed information about each

stormwater treatment option can be found in CWP 2007. Some of the basic differences are compared in **Table 7.6** below.

Table 7.6. Stormwater Treatment Options for Retrofitting

| Stormwater Treatment Option | How It Works |
|--|---|
| Extended Detention (BMP #15)* | This option relies on 12-24 hour detention of stormwater runoff after each rain event within a pond, with portions of the pond drying out in between storm events. Extended detention (ED) allows pollutants to settle out, and if enough storage is available, can also provide downstream channel protection. |
| Wet Ponds (BMP #14)* | Wet ponds consist of a permanent pool of standing water. Runoff from each new storm enters the pond and partially displaces pool water from previous storms. The pool also acts as a barrier to re-suspension of sediments and other pollutants removed during prior storms. |
| Constructed Wetlands (BMP #13)* | Constructed wetlands are shallow depressions that receive stormwater for treatment. Runoff from each new storm displaces runoff from previous storms, and the residence time of several days to weeks allows multiple pollutant removal processes to operate. |
| Bioretention (BMP #9)* | Bioretention is an innovative urban stormwater practice that uses native forest ecosystems and landscape processes to enhance stormwater quality. Bioretention areas capture sheet flow from impervious areas and treat the stormwater using a combination of microbial soil processes, infiltration, evapotranspiration, and plants. |
| Filtering Practices (BMP #12)* | Filter practices function by filtering runoff through an engineered media and collecting treated runoff in an underdrain. The media may consist of sand, soil, compost, or a combination of these. |
| Infiltration Practices (BMP #8) | An infiltration trench is a rock-filled chamber with no outlet that receives stormwater runoff. Stormwater runoff passes through some combination of pretreatment measures, such as a swale or sediment basin, before entering the trench where it infiltrates into the soil. |
| Swales (BMP #'s 3, 10 and 11)* | Swales are a series of engineered, vegetated, open channel practices that are designed to treat and attenuate stormwater runoff for a specified water quality volume. |
| Other Retrofit Treatment (BMP #'s 1, 2, 4, 5, 6, 7 and 9)* | These on-site practices provide treatment of roof runoff using rain gardens, rain barrels, vegetated roofs, cisterns, stormwater planters, dry wells, or permeable pavers. |
| <p>* Practice specifications can be found on the Virginia Stormwater BMP Clearinghouse web site at http://www.vwrrc.vt.edu/swc/NonProprietaryBMPs.html.</p> <p>NOTES: See Chapter 8 for more specific descriptions of these BMPs. More specific and thorough guidance about stormwater treatment retrofits can be found in CWP (2007a).</p> | |

Source: Adapted from CWP, 2007a

More specific and thorough guidance about conveyance system retrofits can be found in CWP (2007a).

Modifications to Existing Facilities. Existing stormwater management facilities originally designed for channel protection or flood control can be modified or reconfigured for water quality mitigation purposes or increased hydrologic benefit. Older detention facilities offer the greatest opportunity for this type of retrofit (**Figure 7.10** below). Traditional dry detention basins can be modified to become extended detention basins, wet ponds, or stormwater wetlands for enhanced pollutant removal. This is one of the most common and easily implemented retrofits

since it typically requires little or no additional land area, utilizes an existing facility for which there is already some resident acceptance of stormwater management, and involves minimal impacts to environmental resources.

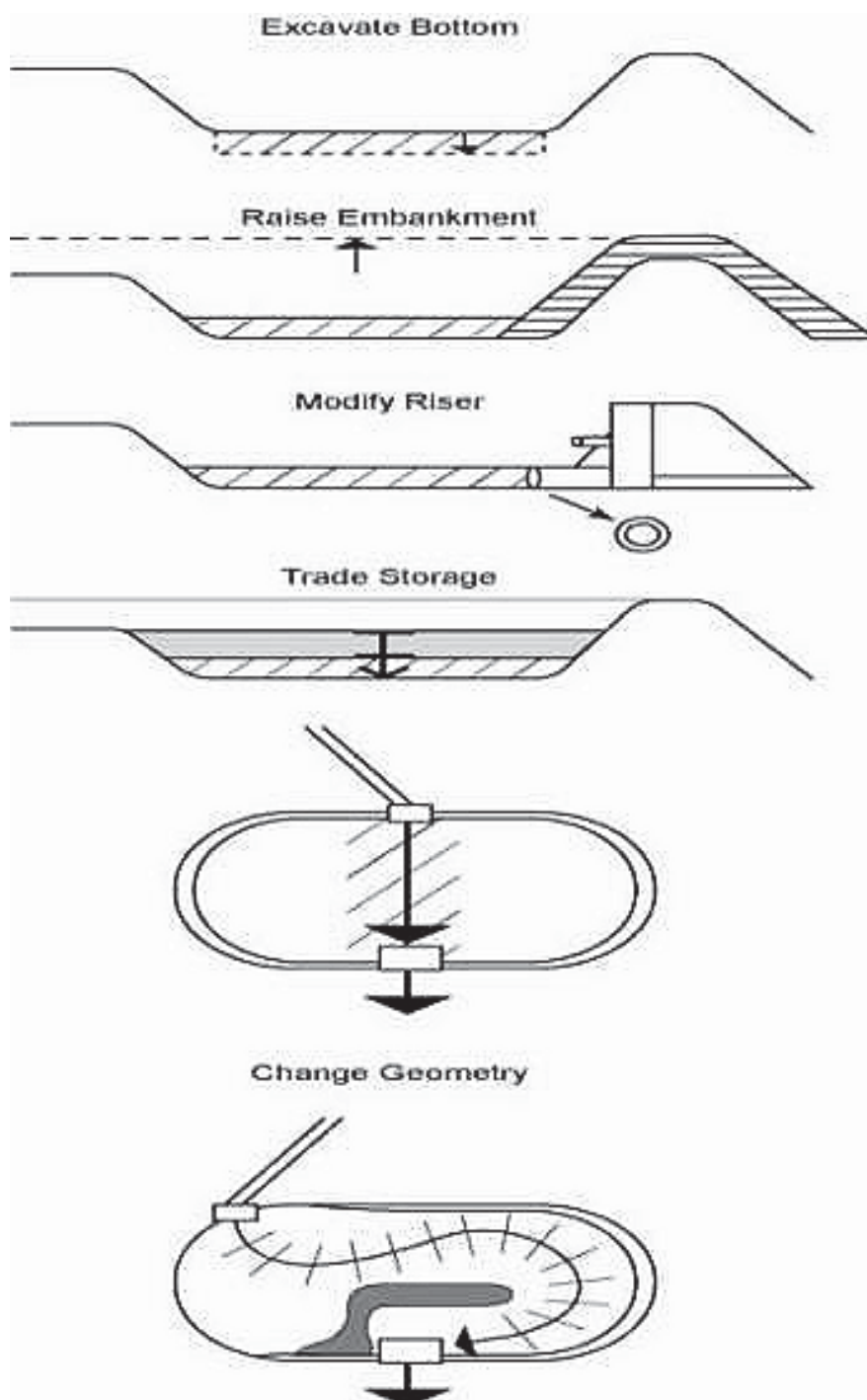


Figure 7.10. Some Modifications That Can Be Made to Existing Detention Ponds.

Source: CWP 2007

Specific modifications to existing detention basins for improved water quality mitigation are summarized as follows (Sources: CT DEP, 2004, adapted from Claytor, CWP, 2000; Pennsylvania Association of Conservation Districts et al., 1998; and NJDEP, 2000):

- Excavate the basin bottom to create more permanent pool storage
- Raise the basin embankment to obtain additional storage for extended detention
- Modify the outfall structure to create a two-stage release to better control small storms while not significantly compromising flood control detention for large storms
- Increase the flow path from inflow to outflow and eliminate short-circuiting by using baffles, earthen berms, or micro-pond topography to increase residence time of water in the pond and improve settling of solids
- Replace paved low-flow channels with meandering vegetated swales
- Provide a high flow bypass to avoid resuspension of captured sediment/pollutants during high flows
- Eliminate low-flow bypasses
- Incorporate stilling basins at inlets and outlets and sediment forebays at basin inlets
- Re-grade the basin bottom to create a wetland area near the basin outlet or re-vegetate parts of the basin bottom with wetland vegetation to enhance pollutant removal, reduce mowing, and improve aesthetics
- Create a wetland shelf along the perimeter of a wet basin to improve shoreline stabilization, enhance pollutant filtering, and enhance aesthetic and habitat functions
- Create a low maintenance “no-mow” wildflower ecosystem in the drier portions of the basin

Stormwater detention basin retrofits should include an evaluation of the hydraulic characteristics and storage capacity of the basin to determine whether available storage exists for additional water quality treatment. Dry pond locations are typically easy to identify on fine-resolution aerial photos (**Figure 7.11**). A typical retrofit of an existing detention basin is shown in **Figure 7.12** below.



Figure 7.11. Dry Ponds Are Easy to Identify on Fine-Resolution Aerial Photos

Source: CWP, 2007

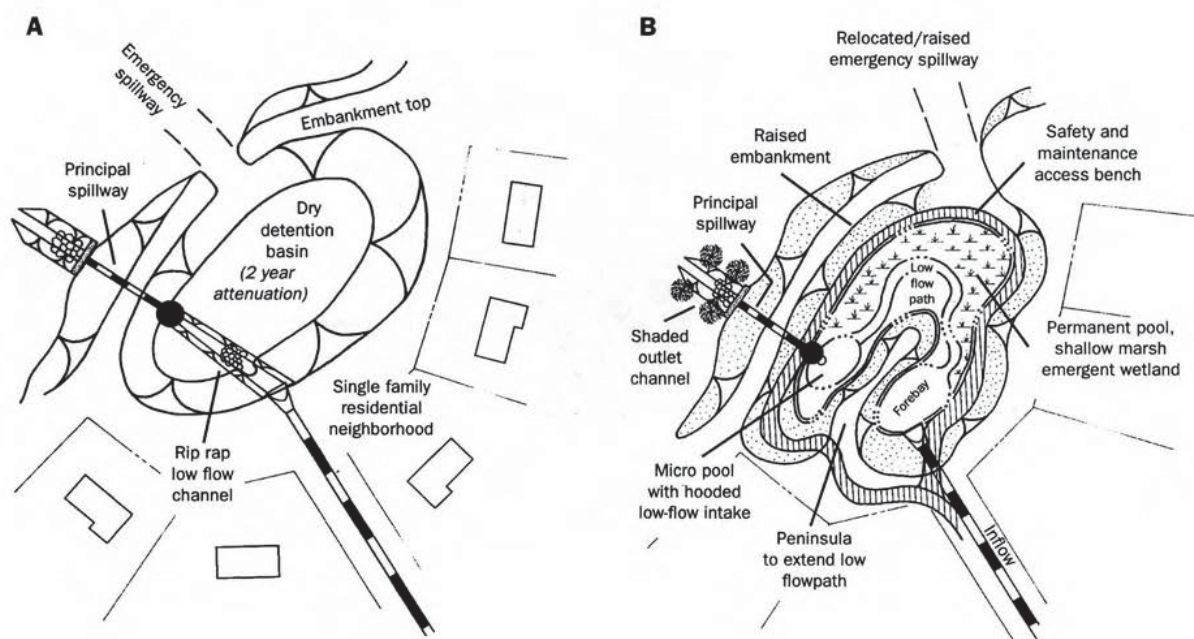


Figure 7.12. Stormwater Retrofit of an Existing Dry Detention Basin

Source: Claytor, CWP, 2000; CT DEP, 2004

Additional enhancements to existing BMPs include rehabilitating failed infiltration practices, adding bioretention or filtering to ponds, and increasing the treatment volume, flow path, retention time, or wetland elements of existing BMPs.

7.7.4. Storm Drain Outfalls

New stormwater treatment practices can be constructed at the outfalls of existing drainage systems. The new stormwater treatment practices are commonly designed as off-line devices to treat the water quality volume and bypass larger storms. Water quality swales, bioretention, sand filters, constructed wetlands, and wet ponds are commonly used for this type of retrofit, although most stormwater treatment practices can be used for this type of retrofit given enough space for construction and maintenance. Manufactured, underground treatment devices are also commonly installed as off-line retrofits at or upgradient of stormwater outfalls. Velocity dissipation devices such as plunge pools and level spreaders can also be incorporated into the retrofit design.

This retrofit creates new treatment adjacent to the stream corridor near the terminus of an existing storm drain outfall. Outfall retrofits are designed off-line by splitting flow from the existing storm drain pipe (or ditch) and diverting it to a stormwater treatment area formed by an existing depression, excavation or constructed berm (**Figure 7.13** below). A flow splitter allows larger storms to remain in the existing pipe (or ditch) and bypass the retrofit. Typical stormwater treatment options at outfall retrofits are a combination of extended detention, pond or constructed wetland storage (**Figure 7.14** below). Constructed wetlands are preferred in floodplains where groundwater elevations are high and space is available. Bioretention may also work if the outfall has no dry weather flow and a small contributing drainage area (**Figure 7.15** below).

Outfall retrofits are ideal because they are close to the stream and maximize the upland drainage area treated. In addition, their offline location usually means fewer stream permitting problems. Finally, outfall retrofits only need to be designed to provide the desired storage for water quality and/or channel protection; larger flood flows bypass the retrofit. More specific and thorough guidance about outfall retrofits can be found in CWP (2007a).

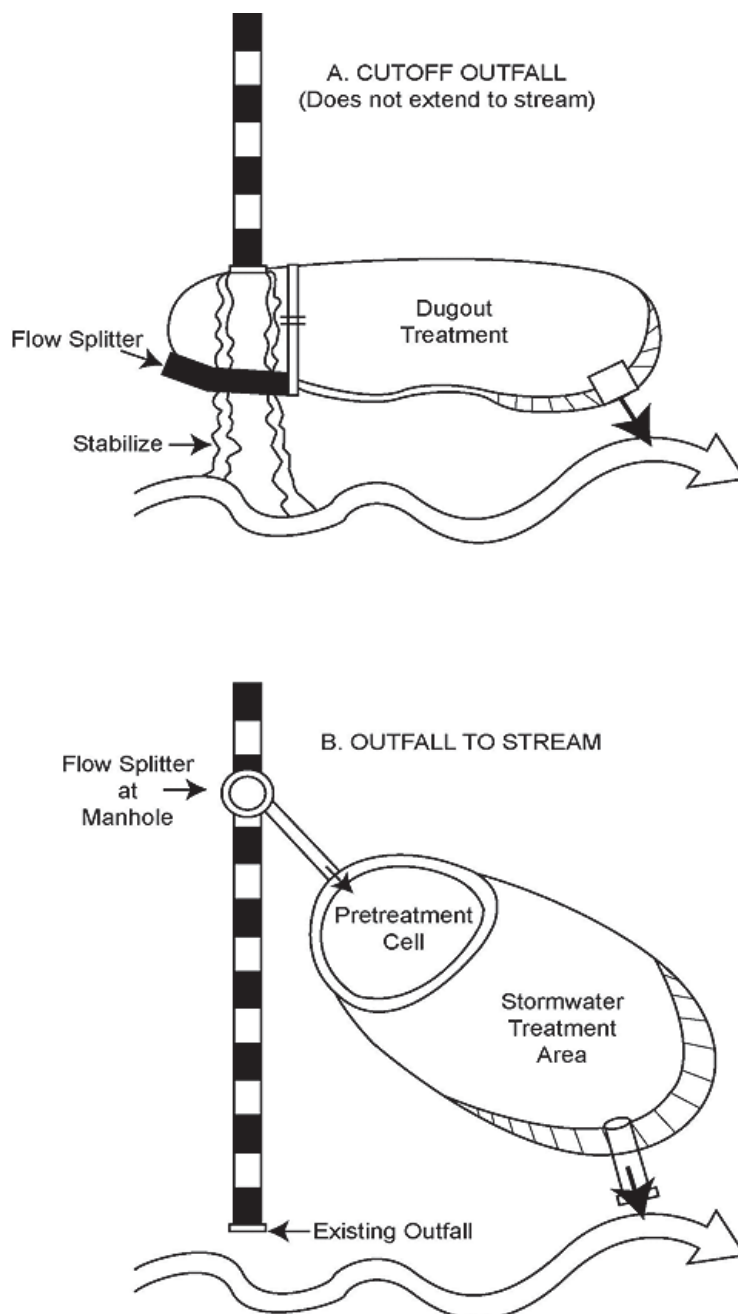


Figure 7.13. Two strategies for outfall retrofits in the stream corridor

Source: CWP, 2007a



Figure 7.14. Example of a “cutoff” outfall discharging well away from the stream to a wetland area (out of picture)

Source: CWP, 2007

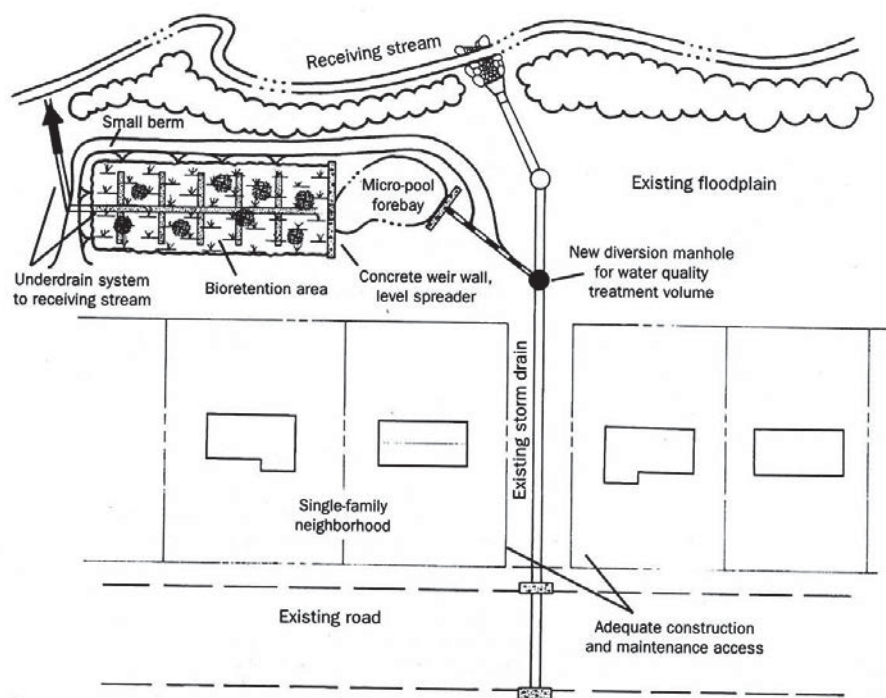


Figure 7.15. Typical Stormwater Retrofit at an Existing Storm Drain Outfall Directly to a Stream. Source: CWP, 2007

7.7.5. Storage Above Roadway Crossings

Road crossings can be modified to provide temporary water quality storage on the upstream side of an existing road culvert. Storage can be obtained by installing a new embankment above the crossing to get “free” storage (**Figure 7.16**). The new embankment would protect the roadway embankment from seepage effects. Available storage can also be increased by excavating areas adjacent to the upstream channel. In general, road crossing retrofits should be applied to non-perennial stream channels to avoid permitting problems (i.e., zero and first order streams). Otherwise, road crossing retrofits can be complicated because various environmental permits and landowner approvals may be needed to construct them.

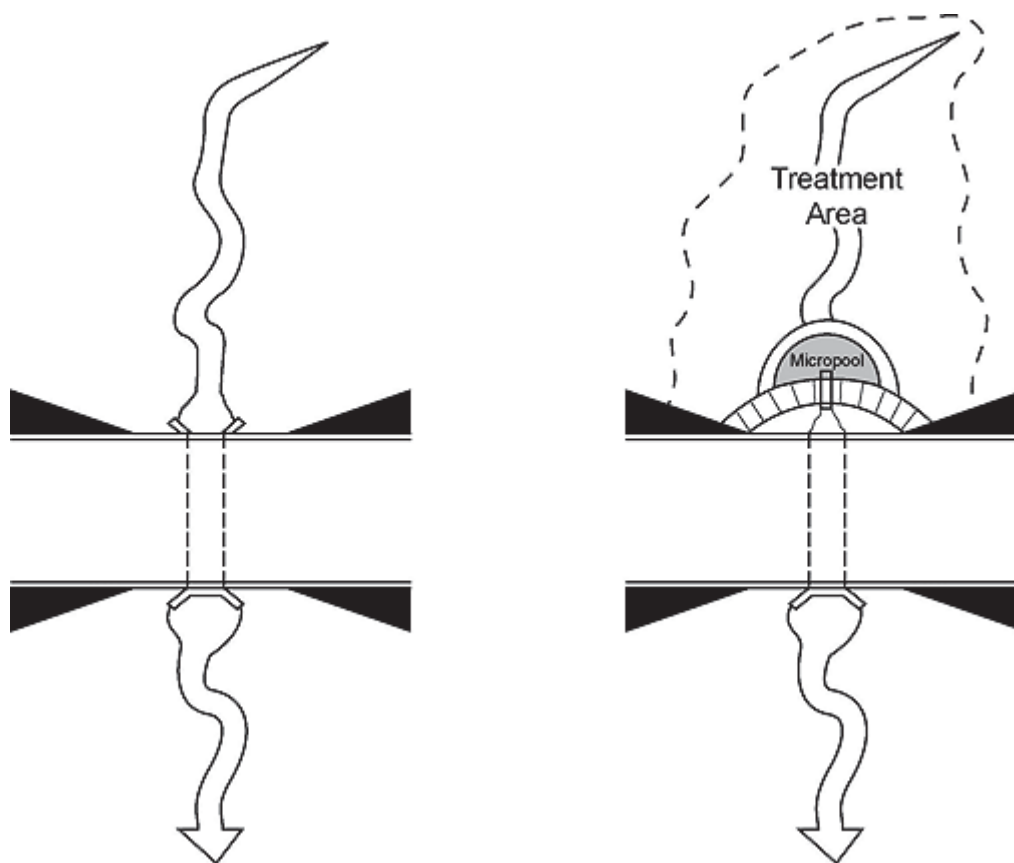


Figure 7.16. Strategy for getting free storage above a road crossing

Source: CWP, 2007

A control structure would normally be installed through the new embankment that creates an upstream micropool (**Figure 7.17** below). The control structure typically consists of a gabion or concrete weir or a riser/barrel. The micropool has a small permanent pool sized to be at least 10% of the total T_v . Extended detention, constructed wetlands and wooded stormwater wetlands are recommended treatment options for road crossing retrofits (see **Figure 7.18** below). Road crossings may also contain enough storage to provide channel protection storage. Crossing retrofits are ideal because they take advantage of free upstream storage, which reduces excavation costs. Opportunities for road crossing retrofits are easy to find in GIS systems when the road network and drainage layers are superimposed (**Figure 7.19** below).

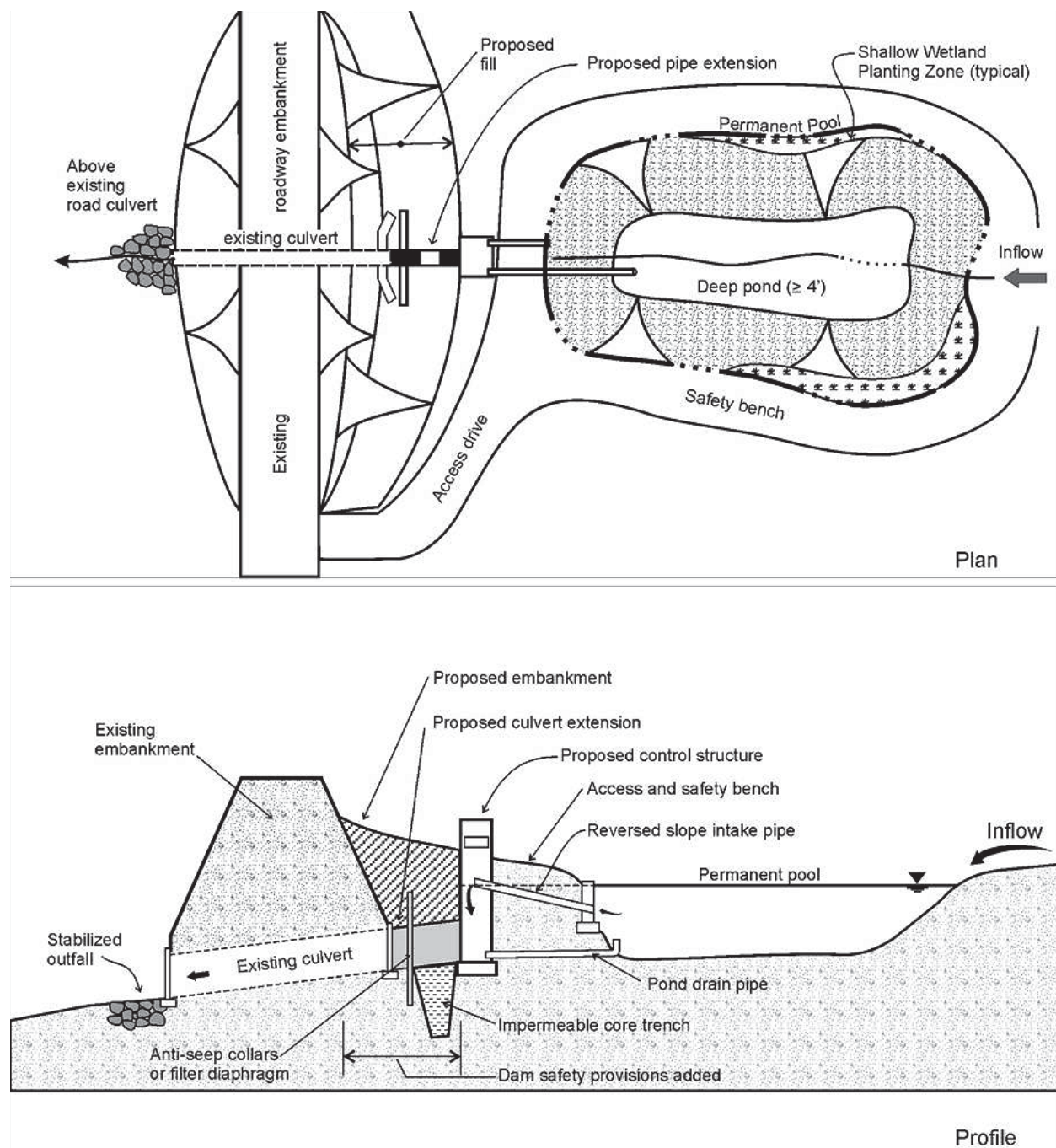


Figure 7.17. Typical plan and profile of crossing retrofit showing secondary embankment
Source: CWP, 2007



Figure 7.18. Wooded wetlands are a preferred stormwater treatment option for crossing retrofits
Source: CWP, 2007



Figure 7.19. Crossing retrofits are easy to find when road network and drainage layers are superimposed
Source: CWP, 2007

Ideal Conditions for Crossing Retrofits

The best situation for a road crossing retrofit is when:

- Ideally, the existing road culvert was already designed as a principal spillway pipe.
- The existing culvert has sufficient hydraulic capacity to pass desired storm flows.
- Upstream land is in public ownership.
- Channel has ephemeral (wet weather) flow only (e.g., zero or first order stream – less likely to require permits).
- Upstream channels are low gradient, are connected to the floodplain, and have short streambanks.
- The retrofit is timed to coincide with scheduled repair/replacement of the existing culvert. In such cases:
 - Avoid using anti-seep collars, which can actually promote failures.
 - Instead, a concrete cradle and pipe joints with gaskets are recommended.
- The retrofit is upstream of a proposed stream restoration or wetland mitigation project.

More specific and thorough guidance about crossing retrofits can be found in CWP (2007a).

7.7.6. Highway Rights-of-Way

Open spaces associated with highway rights-of-way such as medians, shoulders, and cloverleaf areas also present opportunities to incorporate new stormwater treatment practices. Common treatment practices used in these types of retrofits include vegetated swales, bioretention, constructed wetlands, and extended detention ponds. Traffic, safety, and maintenance access are important considerations for determining appropriate locations for highway right-of-way retrofits.

Highways contain un-used land within their right-of-way where storage can be obtained by diverting highway runoff into a depression or excavated area. Highways frequently cross local drainage divides, which reduces contributing drainage area and makes the corresponding T_v storage more manageable. In most cases the contributing drainage area to a highway retrofit is less than 10 acres. The most common stormwater treatment options for highway retrofits are ponds and constructed wetlands, although linear bioretention and dry swales may also be feasible in wider medians and rights-of-way (**Figure 7.20** below). In general, infiltration is not recommended as a stormwater treatment option, unless there is enough pretreatment to fully capture and contain a 10,000 gallon spill.

Ideal Conditions for Highway Retrofits

The best conditions to shoehorn storage retrofits into the highway system occur at:

- Cloverleaf interchanges
- Depressions created by approach ramps
- Open section drainage within a right-of-way that is wider than 30 feet and located down-gradient from the road and free of utilities
- Drainage leading to bridges that cross streams with extensive floodplains

- Highway drainage that can be diverted to adjacent public land
- Targets of opportunity in highway widening/realignment construction projects



Figure 7.20. Highway Corridors Present Numerous Retrofit Opportunities. Source: CWP, 2007

Potential highway retrofit sites can be found using several methods. The quickest is to visually examine aerial photos, since major highway features tend to really stand out (**Figure 7.21**). A more systematic method is search existing local, state or federal highway right-of-way GIS layers against open land and the stream network. The combined land area in open space and right of way should generally meet a minimum acreage threshold of one acre. Most highway agencies have good maps of their road drainage, so try to get copies to take into the field (**Figure 7.22**). These maps should be analyzed to find any existing highway stormwater treatment practices that might be suitable for retrofitting. More specific and thorough guidance about highway right-of-way retrofits can be found in CWP (2007a).



Figure 7.21. Highway retrofits really stand out in aerial photos, although highway drainage does not. Source: CWP, 2007

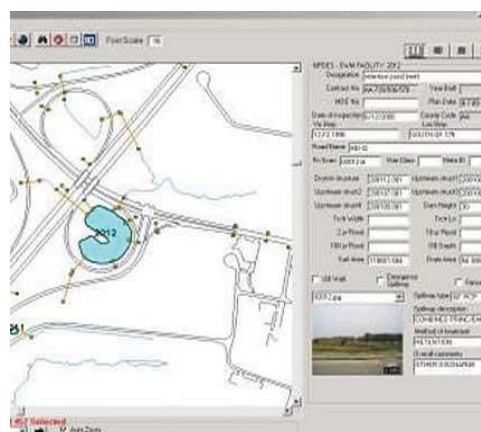


Figure 7.22. Many highway agencies have good GIS data for their stormwater infrastructure. Source: CWP, 2007

7.7.7. Individual Streets

This group of on-site retrofits provides stormwater treatment within the roadbed or right of way of individual streets. A wide range of retrofit strategies can be employed depending on whether the street has open or closed drainage:

- Install stormwater treatment within open section drainage
- Convert enclosed drainage into open section and install stormwater treatment practices
- Divert stormwater for surface treatment before it enters the storm drain
- Make storm drain pipes less efficient at delivering stormwater by promoting infiltration in the storm drain pipe.

Stormwater treatment options for open section street retrofits include dry swales, grass channels, bioretention cells and wet swales. Streets with closed drainage may utilize street bioretention, expanded tree pits, cul-de-sac bioretention, catch basin inserts or perforated storm drain pipes. **Figure 7.23** illustrates many different ways stormwater treatment can be applied to street retrofits.

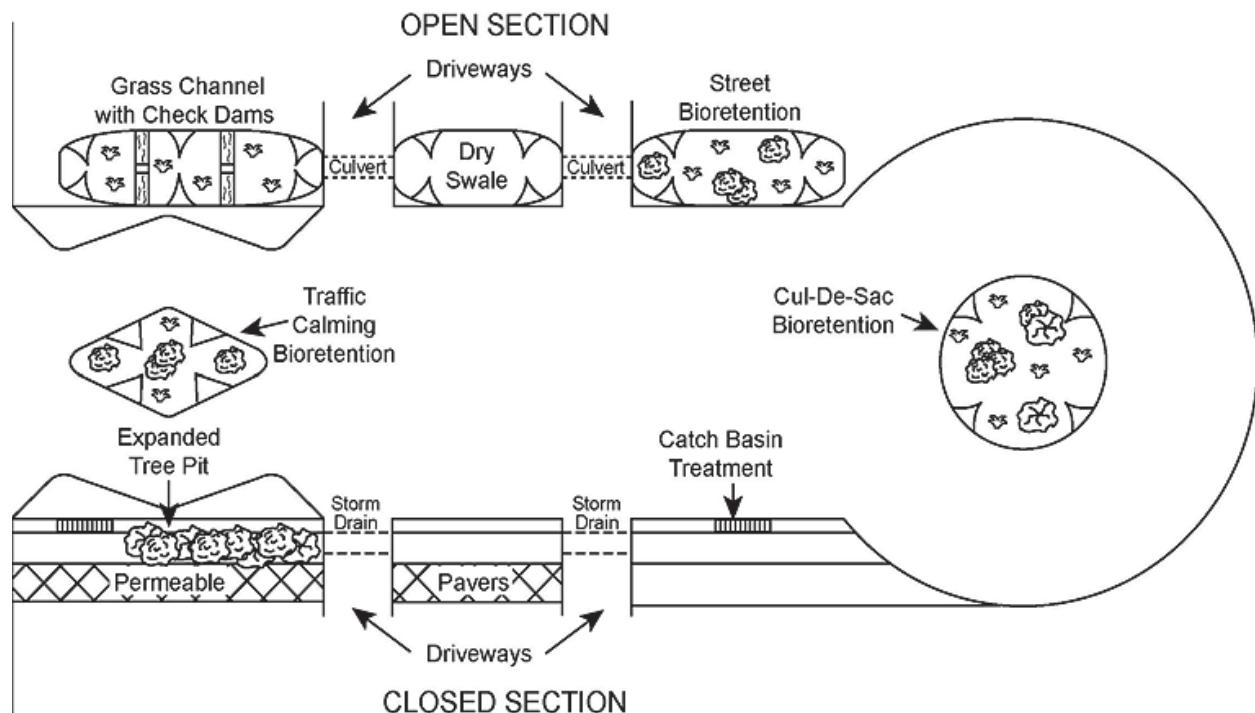


Figure 7.23. Retrofit strategies depend on whether the street has open or enclosed drainage

Source: CWP, 2007a

Streets are a significant urban pollutant source area and act as the primary conduit to move stormwater runoff from rooftops, lawn and driveways. Street retrofits treat stormwater near the source, improve neighborhood appearance, calm traffic and act as a focal point to educate adjacent residents about stormwater quality. Creative techniques to retrofit streets are shown in **Figures 7.24 through 7.27**.



Figure 7.24. Bioretention in Street Median
Source: CWP, 2007a



Figure 7.25. Curb Cut to Rain Garden
Source: CWP, 2007a



Figure 7.26. Bioretention in Traffic Calming Areas. Source: CWP, 2007a



Figure 7.27. Designers need to find creative ways to pass runoff across driveways and sidewalks. Source: CWP, 2007a

Ideal Conditions for Street Retrofits

Most communities maintain hundreds or even thousands of residential street miles (Law, 2006). Key suitability factors for street retrofits include:

- Streets classified as having a moderate to severe pollution severity, as measured by field surveys.
- Neighborhoods that request traffic calming devices to slow residential speeding
- Streetscaping projects or neighborhood revitalization efforts where street drainage can be modified
- Bundling retrofits as part of upcoming water and/or sewer rehabilitation projects
- Wider streets that serve large lots (1/2 acre lots and up)
- Wide street right of ways that provide room for stormwater treatment options
- Streets where utilities are located underneath the pavement or on only one side of the street

More specific and thorough guidance about individual street retrofits can be found in CWP (2007a).

7.7.8. Parking Lots

Parking lots can be ideal candidates for a wide range of stormwater retrofits. Potentially applicable retrofits include site planning techniques and small-scale management measures to reduce impervious coverage and increase infiltration, as well as a variety of larger, end-of-pipe treatment practices. Redevelopment of older commercial properties, which were often designed with oversized parking lots and almost 100 percent impervious coverage, is one of the most common and environmentally beneficial opportunities for parking lot stormwater retrofits.

Alternative site design and LID management practices are well suited to existing developed areas because most of these practices use a small amount of land and are easily integrated into existing parking areas. Examples of these parking lot stormwater retrofits include:

Incorporating Bioretention Into Parking Lot Islands and Landscaping. Parking lot islands, landscaped areas, and tree planter boxes can be converted into functional bioretention areas and rain gardens to reduce and treat stormwater runoff.

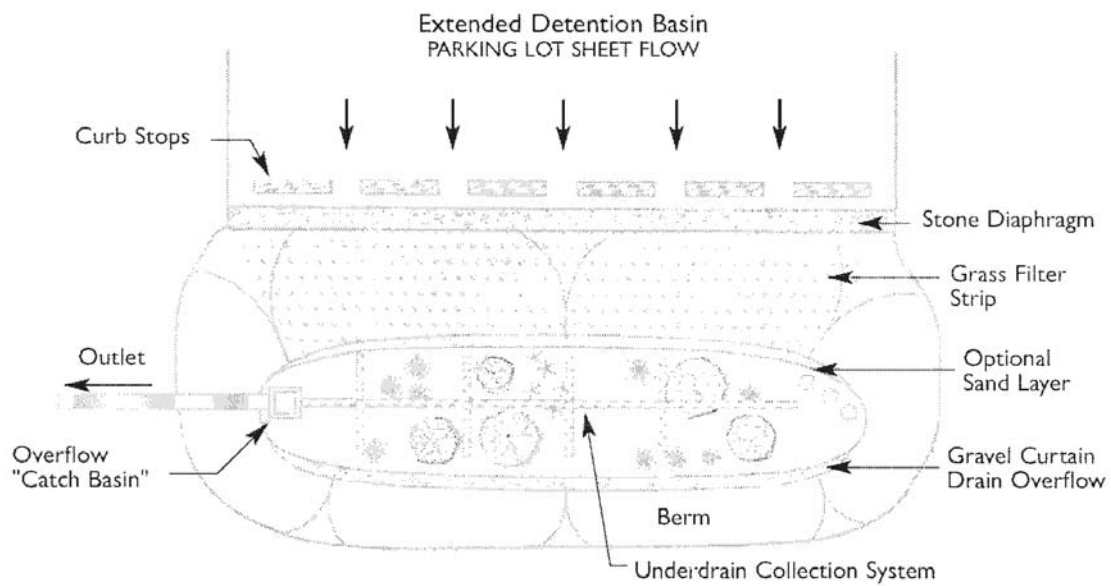
Removing Curbing and Adding Slotted Curb Stops. Curbs along the edges of parking lots can sometimes be removed or slotted to re-route runoff to vegetated areas, buffer strips, or bioretention facilities. The capacity of existing swales may need to be evaluated and expanded as part of this retrofit option.

Infiltrating Clean Roof Runoff From Buildings. In some instances, building roof drains connected to the stormwater drainage system can be disconnected and redirected to vegetated areas, buffer strips, bioretention facilities, or infiltration structures (dry wells or infiltration trenches).

Incorporating New Treatment Practices at the Edges of Parking Lots. New stormwater treatment practices such as bioretention, sand filters, and constructed wetlands can often be incorporated at the edges of large parking lots.

Use of Permeable Paving Materials. Existing impermeable pavement in overflow parking or other low-traffic areas can sometimes be replaced with alternative, permeable materials such as modular concrete paving blocks, modular concrete or plastic lattice, or cast-in-place concrete grids. Site-specific factors including traffic volumes, soil permeability, maintenance, sediment loads, and land use must be carefully considered for the successful application of permeable paving materials for new development or retrofit applications.

Figure 7.28 below depicts some of the parking lot stormwater retrofits described above.



Parking Lot Infiltration

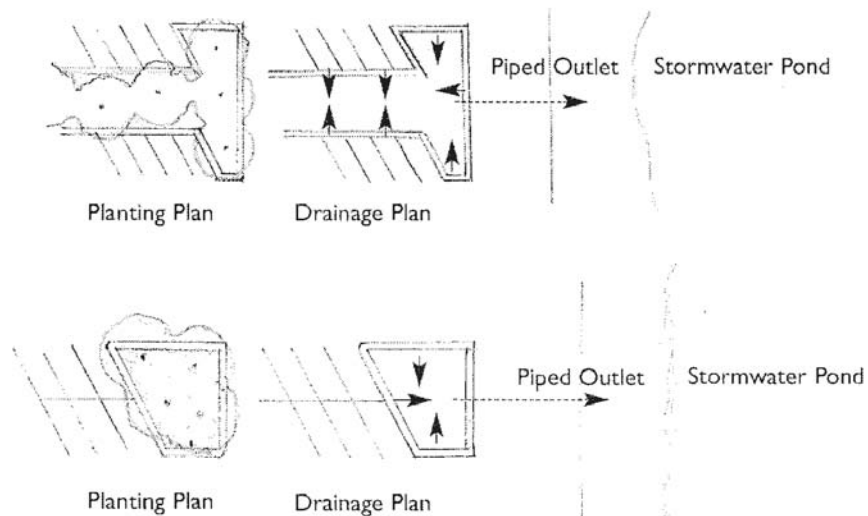
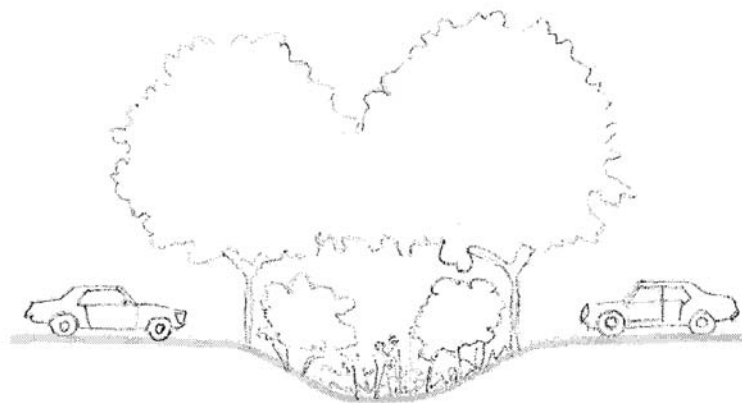


Figure 7.28. Parking Lot Stormwater Retrofit Schematics

Source: CT DEP, 2004; Metropolitan Council, 2001 (Adapted from VBWD, 2000); and NYDEC, 2001

7.7.8.1. Large Parking Lot Retrofits

Large parking lots are a good retrofit opportunity to treat runoff quality. *Large parking lots are defined as five acres or greater in size, including any connected rooftops (see Figure 7.29).* Common examples include lots serving municipal buildings, high schools, regional shopping malls, stadiums, auto dealerships, airports, commuter lots, hospitals and big box retail stores. Larger parking lots are normally served by extensive storm drain systems and contain numerous inlets, underground pipes and outfalls.

This retrofit strategy excavates centralized treatment storage in unutilized land located down-gradient of the lot (**Figure 7.30**). Common stormwater treatment options include wet ponds, extended detention, ponds, constructed wetlands or a large bioretention area. Centralized retrofits are not the only retrofit strategy for parking lots, but the centralized retrofit strategy is generally more cost-effective on a per acre treated basis than an on-site strategy. Large parking lots are an ideal retrofit because they generate more stormwater runoff and pollutants on a unit area basis than any other land use in a subwatershed.



Figure 7.29. Large Parking Lots are a Key Retrofit Target. Source: CWP, 2007a



Figure 7.30. Down-Gradient Open Land Reserved in Setbacks Is Ideal for Treatment. Source: CWP, 2007a

Ideal Conditions for Large Parking Lot Retrofits

Parking lots built in the last few decades are good retrofit opportunities since local codes often require more generous setbacks for screening, landscaping and noise reduction. Recently developed suburban commercial zones are only about 70% impervious, suggesting that a decent fraction of the site may be available for surface treatment (Cappiella and Brown, 2001). Other good retrofit situations are:

- Parking lots serving large institutions, corporate campuses and colleges that tend to have even lower percentage of impervious cover for the whole site.
- Municipally-owned parking lots such as commuter lots, park access, and schools adjacent to open areas
- Industrial parking lots designated as stormwater hotspots
- Any parking lot served by an existing stormwater detention pond (use SR-1)

Restoration Alternatives at Large Parking Lots

Even if a storage retrofit is not feasible, it may still be possible to install other restoration practices inside the parking lot or along its margins, such as:

- *Reforestation* in open spaces, parking lot islands and setbacks using the planting methods outlined in Cappiella *et al.* (2006a).
- *Pollution prevention practices*, particularly when the lot is used for vehicle storage or is frequently resealed
- *Regular vacuum sweeping and litter control* to keep gross solids and trash from entering the storm drain system.

7.7.8.2. Small Parking Lot Retrofits

This on-site retrofit strategy treats the quality of runoff from existing parking lots less than five acres in area. Surface retrofits can be installed within the parking lot, along its perimeter, or in adjacent pervious areas (**Figure 7.31**).

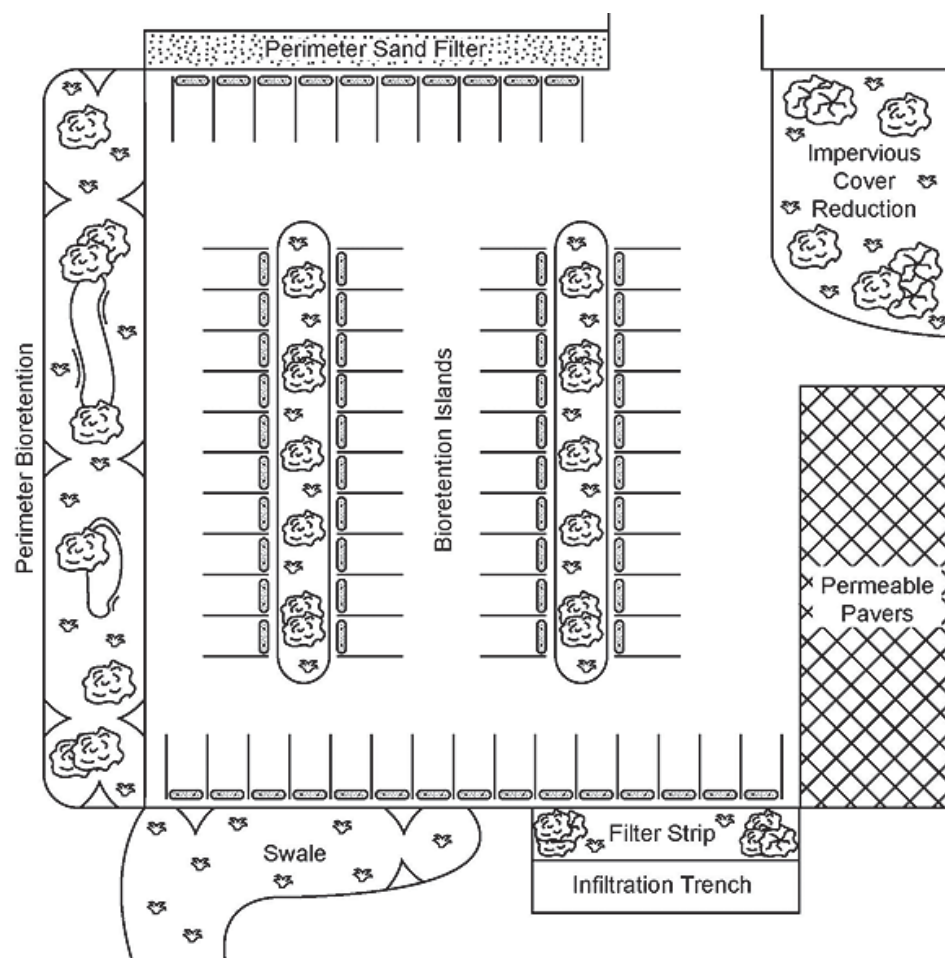


Figure 7.31. Many different retrofit strategies can be employed to retrofit parts of a smaller lot.

Source: CWP, 2007a

Small parking lots are generally quite easy to spot on aerial photographs or GIS data layers (**Figure 7.32**). A more systematic approach may restrict the search to parking lots in municipal or institutional ownership where permission to retrofit may be easier to get. Otherwise, the feasibility small parking lot retrofits is normally determined in the field.



Figure 7.32. Orthophotos can help find small parking lots and for concept sketches.

Source: CWP, 2007a

A wide range of stormwater treatment options can be adapted for this retrofit, including:

- Impervious Cover Reduction
- Permeable Pavers
- Bioretention Islands
- Perimeter Bioretention
- Perimeter Sand Filter
- Filter Strips
- Infiltration
- Dry Swales

Parking lots are an ideal location for on-site retrofits since they generate extremely high unit area runoff volumes, pollutant loads and temperature spikes. Parking lot retrofits also have great demonstration value due to their high visibility. **Figure 7.33** below presents numerous examples of small parking lot retrofit techniques.

Ideal Conditions for Small Parking Lot Retrofits

The best conditions to retrofit small parking lots are when:

- Communities retrofit a municipally owned parking lot as a demonstration project
- New parking lots are constructed as part of redevelopment or infill projects
- Existing parking lots are slated for resurfacing, reconfiguration or renovation (their normal design life is about 15 to 25 years)
- Local stormwater regulations trigger water quality control at time of lot renovation or rehabilitation
- Parking lots were built with generous landscaping, open space, screening or frontage setbacks
- Parking lots are not fully utilized because they were designed using excessive parking demand ratios

Alternative Restoration Practices for Small Parking Lots

Even if an on-site retrofit is not feasible, the following restoration practices may still be viable:

- *Tree planting* in parking islands, lot margins and setbacks.
- *Vacuum sweeping and litter control* in the parking lot.

- *Parking lot pollution prevention practices*, especially for vehicle storage and parking lot maintenance.

More specific and thorough guidance about parking lot retrofits can be found in CWP (2007a).



Figure 7.33. Examples of retrofits employed at small parking lots: (a) permeable pavers; (b) dry swale; (c) perimeter sand filter; (d) grass filter/infiltration trench; (e) filter strip; (f) internal bioretention; (g) underground infiltration; and (h) island bioretention. Source: CWP, 2007a

7.7.9. In-stream Practices in Drainage Channels

Existing (man-made) channelized streams and drainage conveyances such as grass channels can be modified to reduce flow velocities and enhance pollutant removal. Weir walls or riprap check dams placed across a channel create opportunities for ponding, infiltration, and establishment of wetland vegetation upstream of the retrofit (Claytor, Center for Watershed Protection, 2000). In-stream retrofit practices include stream bank stabilization of eroded areas and placement of habitat improvement structures (i.e., flow deflectors, boulders, pools/riffles, and low-flow channels) in impacted natural streams and along stream banks. In-stream retrofits may require evaluation of potential flooding and floodplain impacts resulting from altered channel conveyance, as well as local, state, or federal approval for work in wetlands and watercourses. More comprehensive urban stream and stream corridor restoration practices are beyond the scope of this Manual. Additional sources of information on stream restoration practices are included at the end of this chapter.

7.7.10. Wetland Creation and Restoration

Wetland creation or restoration can partially substitute for lost ecological functions of a destroyed or degraded wetland system in developed areas. Creation or restoration of freshwater or tidal wetlands can improve the pollutant removal, longevity, adaptability, and habitat functions of wetland systems (CT DEP, 1995). Techniques to improve pollutant removal in created or restored wetlands include (Schueler et al., 1992):

- Increasing wetland volume to increase residence time
- Increasing the surface area to volume ratio of the wetland
- Increasing the flow path through the wetland
- Providing energy dissipation and primary sedimentation either prior to the wetland or in a sediment forebay at the wetland inflow locations
- Integrating with other treatment practices such as extended detention

When wetlands are altered through clearing of vegetation, impoundment of water, or dredging, the microhabitats used by many wildlife species are changed or lost. This may result in unsuitable breeding habitat for many amphibians, including vernal pool species. Similarly, created wetlands usually lack the structural diversity, microhabitats, and hydrology to support vernal pool breeding amphibians (Calhoun and Klemens, 2002). Altered and created wetlands often support highly adaptable, widespread, “weedy” species (e.g., bullfrogs or green frogs) that prey upon, or successfully out-compete, vernal pool-breeding amphibians, which reduces or locally eliminates populations of these habitat specialists. Created wetlands that do not have the appropriate habitat often function as “decoy” pools and trap breeding amphibians. Therefore, these wetland creation and restoration techniques should only be implemented with careful consideration of the effects to wetland function and hydrology and in conjunction with applicable local, state, and federal wetland and watercourses regulatory agencies.

7.7.11. Hotspots

These retrofits provide on-site water quality treatment at confirmed stormwater hotspots, defined as any operation that generates higher concentrations of stormwater pollutants and/or has a higher risk of spills, leaks or illicit discharges. Pollution prevention practices such as covering, secondary containment, and employee training should always be considered first. However, when prevention practices are not sufficient to provide full treatment, on-site retrofits are needed to treat the quality of runoff from the stormwater hotspot (**Figure 7.34**).

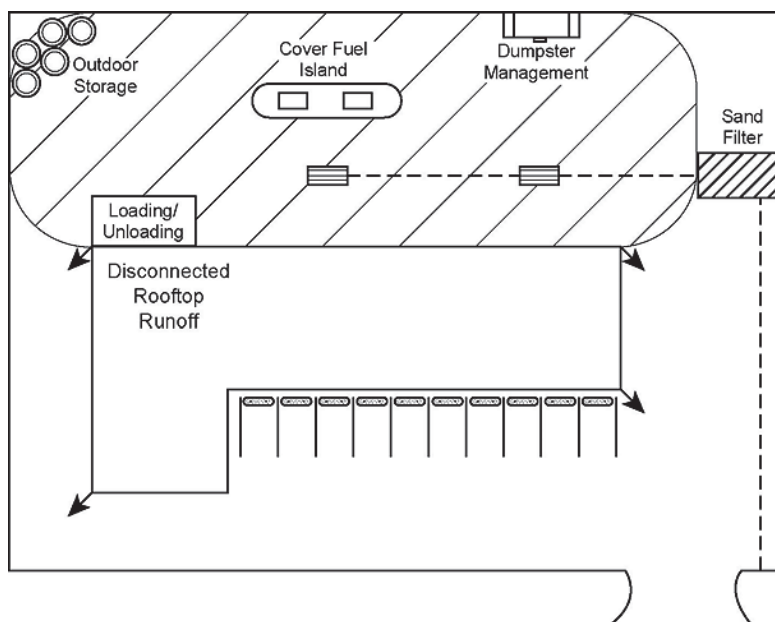


Figure 7.34. Schematic showing typical treatment at hotspot generating areas

Source: CWP, 2007a



Figure 7.35. Filtering Practices Are Preferred for Retrofits at Hotspot Sites. Source: CWP, 2007a

The preferred stormwater treatment option at hotspot operations are filtering practices (**Figure 7.35**). Alternatively, bioretention without exfiltration may be used. The use of infiltration is strongly discouraged due to the risk of groundwater contamination.

Hotspots are good locations for on-site retrofits since they contribute higher stormwater pollutant loads than any other urban source area. Second, many communities have the regulatory authority to compel private landowners to install onsite retrofits to comply with municipal or industrial stormwater requirements.

Ideal Conditions for Hotspot Retrofits

Retrofits should always be considered for any operation:

- Found to be a severe hotspot during a hotspot site investigation
- Covered by an existing industrial stormwater permit or specifically designated as a stormwater hotspot in the local water quality ordinance
- Where site investigation shows that pollution prevention practices alone are not sufficient to remove pollutants in stormwater runoff

Alternative Restoration Projects at Stormwater Hotspots

A nonstructural approach can effectively prevent pollution from many stormwater hotspot operations. CWP 2005a describe pollution prevention practices that can be applied to hotspots:

- Vehicle Maintenance and Repair
- Vehicle Fueling
- Vehicle Washing
- Vehicle Storage
- Loading and Unloading
- Outdoor Storage
- Spill Prevention and Response
- Dumpster Management
- Building Repair and Remodeling
- Building Maintenance
- Parking Lot Maintenance
- Turf Management
- Landscaping/Grounds Care
- Swimming Pool Discharges
- Unique Hotspot Operations

What to Look for When Investigating Hotspots

The team can isolate areas to search for hotspots in the field by reviewing maps depicting commercial, industrial or municipal land use (**Figure 7.36** below). Local knowledge can also be helpful. A more systematic approach for finding hotspot sites involves searching local business databases using standard industrial codes (SIC). Methods for conducting an SIC database search can be found in CWP 2005a. Another approach to find potential stormwater hotspots is to search databases of industrial operations that hold stormwater permits.

Procedures to inspect and rank stormwater hotspots are described in the Hotspot Site Investigation (HSI) component of CWP 2005b. The HIS involves a rapid visual assessment to inspect site operations that may cause a stormwater hotspot. If a site is ranked as a confirmed or severe hotspot, then the crew looks into the "plumbing" at the site to determine whether additional stormwater treatment is needed beyond standard pollution prevention practices.

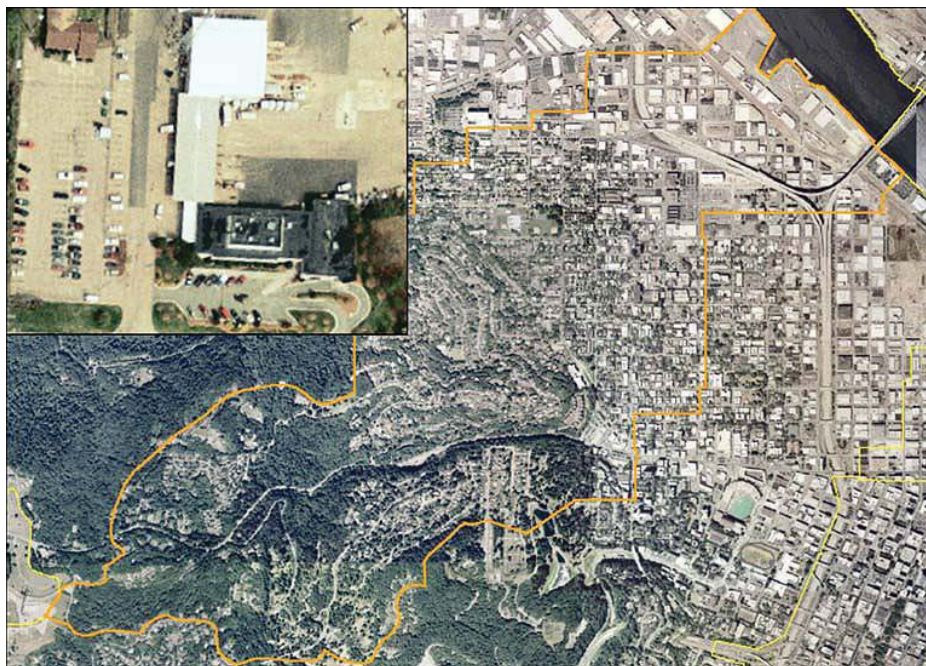


Figure 7.36. Hotspots are too small to find on aerial photos but can be found by searching business databases. Source: CWP, 2007a

Five steps are used to assess the feasibility of on-site treatment at a stormwater hotspot:

- *Define hotspot generating area (HGA)* – which is the area actually generating higher levels of pollutants. The HGA is usually associated with:
 - Vehicle Operations
 - Outdoor Materials
 - Waste Management
 - Physical Plant Maintenance
 - Intensive Turf/Landscaping
- *Evaluate pollution prevention practices* – whether the HGA can be fully treated by non-structural practices such as covering, secondary containment, or employee training. Full treatment is operationally defined as no exposure of the polluting operation to rainfall or runoff. If full treatment cannot be obtained, the crew moves to the next step.
- *Evaluate the hotspot's connection to public storm drain system* – tracing the path of runoff from the HGA as it crosses the site and enters offsite drainage and whether the connections are legal or illicit.
- *Select the stormwater treatment option* – available hydraulic head is usually the key feasibility constraint and is defined as the vertical distance between the elevation of the stormwater inlet and the bottom elevation of the existing storm drain system to which it discharges.
- *Get retrofit design information* – record the details of the selected treatment device, such as the adjusted drainage area, surface and pipe slopes, and notes on soil and subsurface conditions.

More specific and thorough guidance about hotspot retrofits can be found in CWP (2007a).

7.7.12 Individual Rooftops

This group of onsite retrofits captures, stores, treats and then gradually releases runoff from individual rooftops. The goal is to systematically retrofit as many residential and non-residential rooftops as possible within a given subwatershed. The many different ways that rooftops can be retrofitted are portrayed in **Figure 7.37**. A variety of stormwater treatment options can be employed for rooftop retrofits as shown below:

Residential rooftops

- Simple Disconnection
- Rain Barrels
- Rain Gardens
- French Drain/Dry Wells

Non-residential rooftops

- Simple Disconnection
- Rain Gardens
- Stormwater Planters
- Cisterns
- Green Rooftops

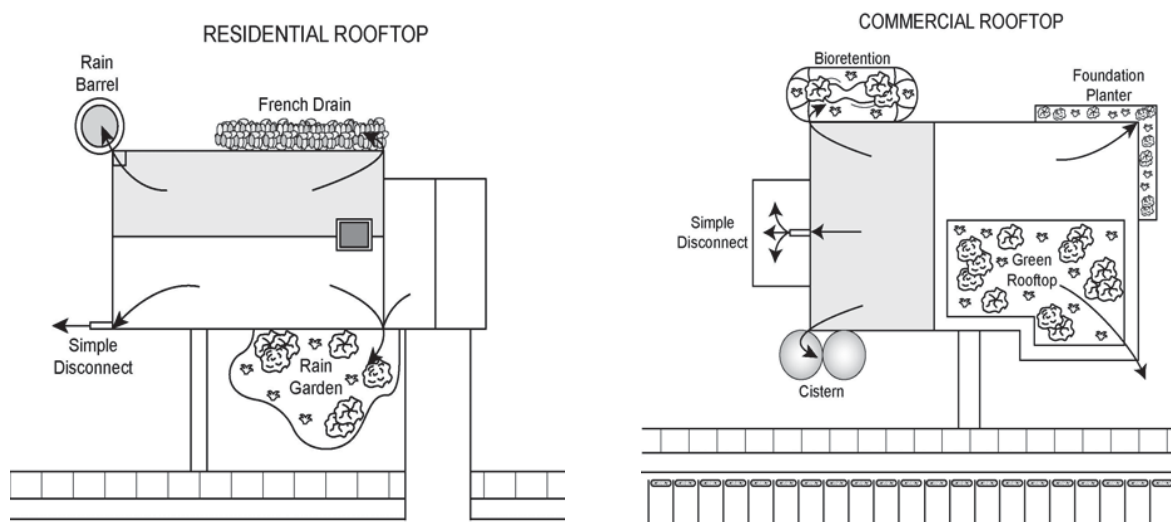


Figure 7.37. A variety of retrofit strategies can be applied to treat the quality of runoff

Source: CWP, 2007a

Examples of rooftop retrofit techniques are shown in **Figures 7.38 through 7.40** below. Rooftop retrofits are ideal when a comprehensive delivery system is developed to implement them on a widespread basis. From a cost-benefit standpoint, it makes more sense to target residential rooftops first since they comprise a greater fraction of subwatershed area and are less expensive on a unit-area treated basis.

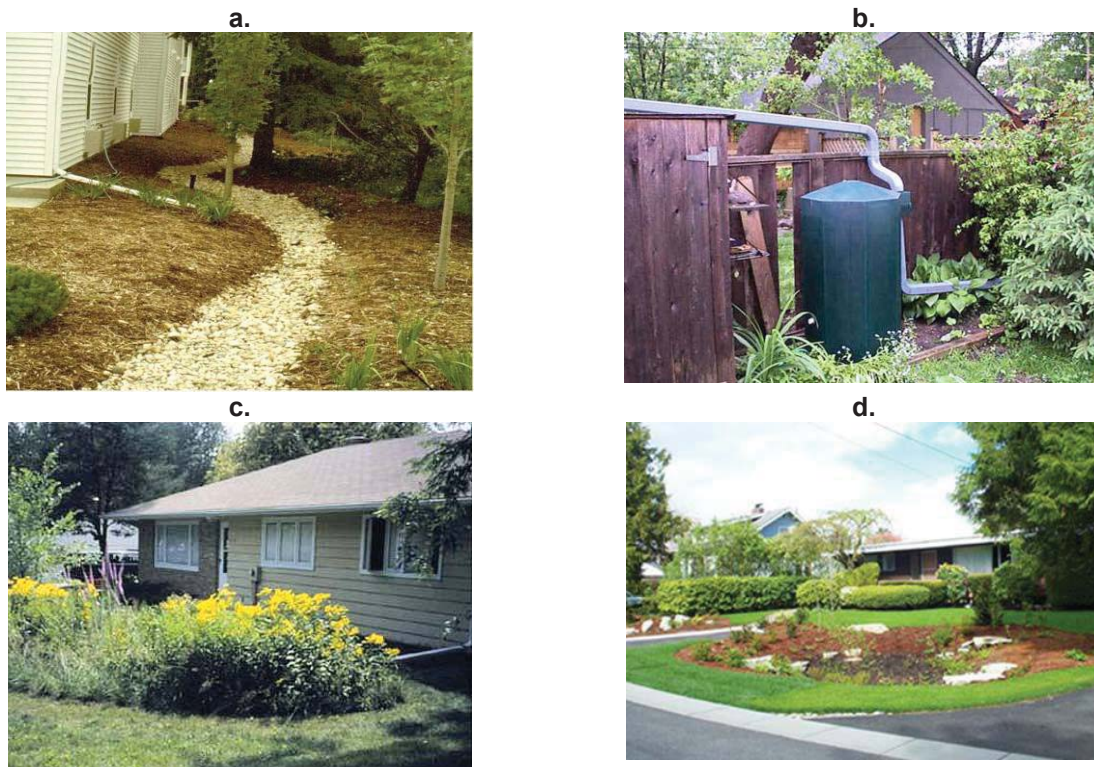


Figure 7.38. Residential rooftops can be treated by (a) french drains, (b) rain barrels, or (c&d) rain gardens. Source: CWP, 2007a

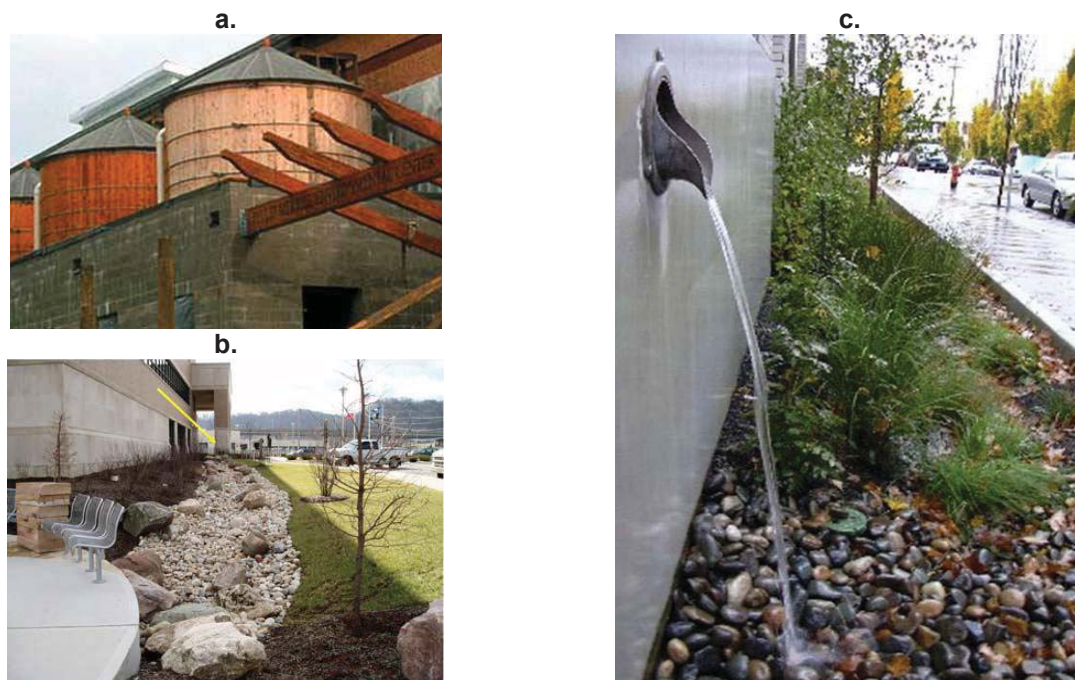


Figure 7.39. Runoff from larger rooftops can be treated in (a) cisterns, (b) infiltration areas, or (c) bioretention planting beds. Source: CWP, 2007a



Figure 7.40. Green rooftops can also treat the quality of runoff from flat rooftops. Source: CWP, 2007a

Rooftop retrofits are particularly well-suited in subwatersheds where runoff reduction is a major restoration goal (e.g., to reduce the volume of stormwater runoff entering a combined sewer system). Retrofitting rooftops for water quality purposes is less effective since rooftop runoff tends to be cleaner than other urban source areas (with the possible exception of metals). On the other hand, incremental rooftop retrofitting can be an effective long range strategy to control runoff in highly urban subwatersheds.

Ideal Conditions for Rooftop Retrofits

The ideal conditions to retrofit residential rooftops are when a neighborhood:

- Has no basements (if infiltration is used)
- Has homes where roof leaders are directly connected to storm drain system
- Is located in a subwatershed where stormwater reductions can reduce combined sewer overflows
- Has a strong neighborhood association, environmental concern or community activism
- Has medium-density residential lot sizes in the 0.25 to 1.0 acre range.

Rooftop retrofits work best in nonresidential settings when:

- The rooftop is being built as part of redevelopment or infill project
- The rooftop is owned or being built by a municipality or a cooperative institution
- The rooftop can discharge to landscaping or open space adjacent to the building
- The rooftop has reached the end of its design life and needs replacement.
- The rooftop is large, flat and directly connected to the storm drain system
- The owner is interested in green building certification

Desktop Searching for Rooftop Retrofits

A search is not very helpful in finding individual rooftop retrofit sites, although the average age and lot size in a neighborhood are worth assessing, since homes built to the same drainage standards tend to have similar retrofit potential. Another GIS search option is to look for specific neighborhoods that deliver stormwater into combined sewers or have historic flooding or drainage problems. Rooftop retrofits alone may not solve these problems, but can play a role in a

larger package of retrofit solutions. A GIS search that defines older commercial, industrial or institutional zones that are near the end of their design life may help find good candidates for non-residential rooftop retrofits. A search of all municipal buildings in a subwatershed may also be warranted to assess their suitability for demonstration retrofits.

More specific and thorough guidance about individual rooftop retrofits can be found in CWP (2007a).

7.7.13. Little Retrofits

Little retrofits are simple on-site practices that treat runoff from directly connected impervious areas less than one acre in size (**Figure 7.41**). Examples include sidewalks, bike paths, driveways, basketball and tennis courts, vacant lots, compacted ball fields, paved play areas, and other surfaces that are impermeable to rainfall. Recommended stormwater treatment options for little retrofits include swales, infiltration, filter strips, impervious cover conversion, impervious cover disconnection and soil compost amendments.



Figure 7.41. Rain Garden Treating Runoff from a Trail
Source: CWP, 2007a

Collectively, small impervious areas comprise less than 5% of total impervious area in a subwatershed. So why bother with little retrofits? The reason is that small impervious areas are easy to retrofit because they are isolated within larger pervious areas. Many small impervious areas fall below minimum area thresholds that trigger stormwater management requirement and were therefore built without consideration for engineered drainage or stormwater practice.

Little retrofits are ideal because they are low cost, require less sophisticated design and can solve localized drainage and erosion problems. In many cases, they can be constructed by watershed

groups, homeowners associations or property managers with minimal engineering background. Furthermore, if a little retrofit doesn't work at a site, reforestation is always a restoration option.

Ideal Conditions for Little Retrofits

The best conditions for little retrofits are when the retrofit:

- Is located on publicly-owned land such as a park or school
- Would serve an educational or demonstration function
- Is in close proximity to a large pervious area
- Would alleviate an existing drainage or erosion problem
- Can take advantage of soils with a high infiltration rate
- Can be linked with a planned reforestation project for the site

7.7.14. Landscapes/Hardscapes

This class of retrofits relies on landscaping to treat stormwater in highly urban settings. Examples include commercial landscaping areas, plazas, waterfronts, urban streetscapes, and pocket parks (**Figure 7.42**). While these urban landscapes occupy a trivial amount of total subwatershed area, they are included here because they represent a great opportunity to demonstrate retrofits in highly visible locations. The basic strategy is to treat stormwater as a landscaping resource and design amenity using innovative practices such as rain gardens, stormwater planters, expanded tree pits or permeable pavers (**Figure 7.43** below).



Figure 7.42. Bioretention Area in a Public Park.

Source: CWP, 2007a



Figure 7.43. Landscape architects can creatively use stormwater as a resource in (a) foundation planters; (b) permeable pavers; (c) bioretention; and (d) stormwater tree pits. Source: CWP, 2007a

Landscape/hardscape retrofits are ideal because they have strong demonstration and education value, are frequently maintained, and may lower landscaping maintenance costs through reduced mowing, greater tree survival, or less irrigation.

Ideal Conditions for Landscape/Hardscape Retrofits

- Commercial, municipal, institutional and urban park settings
- Redevelopment and infill projects
- Public spaces with high exposure
- Area where urban water features are being designed as an amenity
- Downtown central business districts
- Waterfront developments
- Development constructed through public/private partnerships
- Neighborhood beautification and revitalization projects (**Figure 7.44** below)

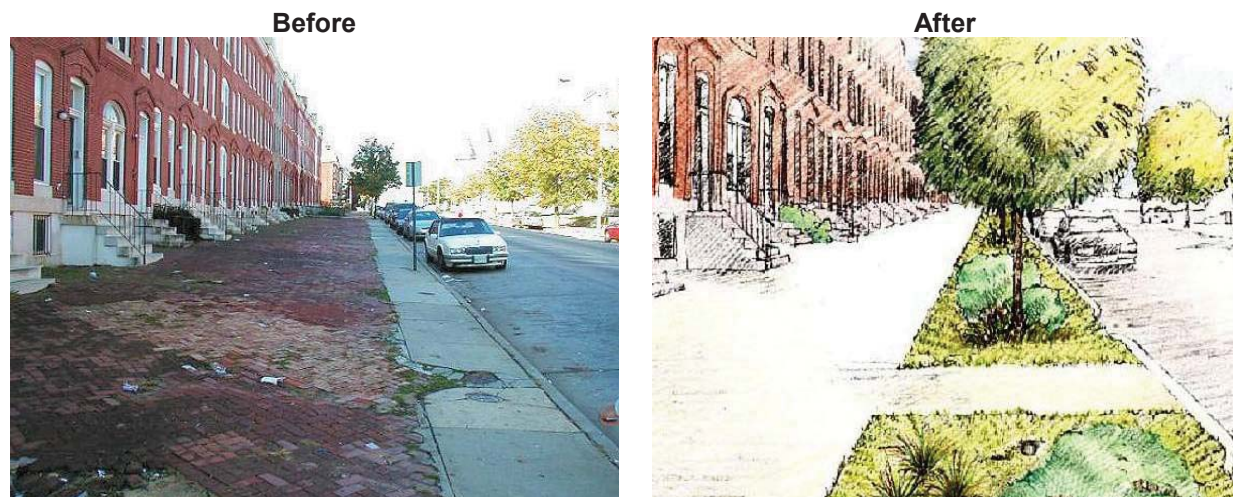


Figure 7.44. Urban foresters can treat stormwater using creative street tree planters.

Source: CWP, 2007a

7.7.15. Underground

Underground retrofits are the on-site retrofit of last resort due to their high cost. They make sense when other on-site retrofits cannot fit on the surface, or land acquisition costs are too high. Underground retrofits are normally restricted to small sites that generate high pollutant loadings discharging to sensitive waters. Common methods of underground treatment are shown in **Figure 7.45** and include:

- Infiltration galleries
- Underground sand filter
- Underground detention pipes
- Multi-chamber treatment train (MCTT)
- Proprietary stormwater treatment devices

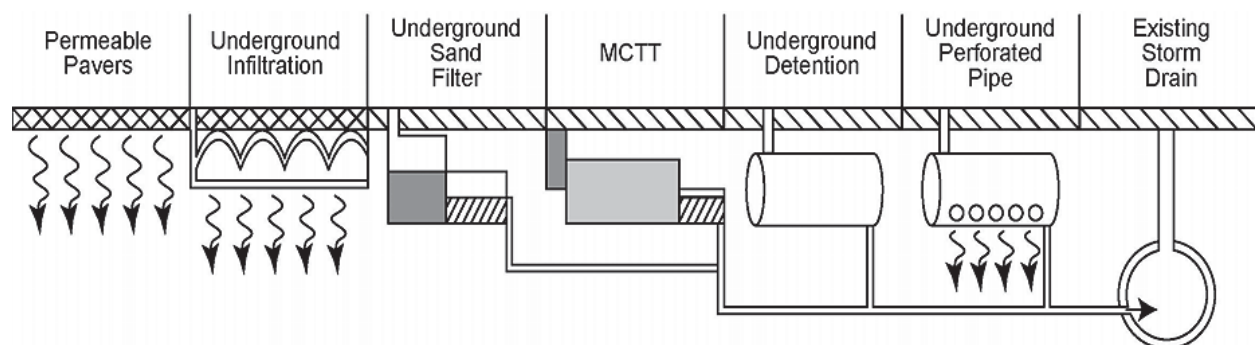


Figure 7.45. Numerous strategies can be used for underground retrofits.

Source: CWP, 2007a

This class of retrofits applies to ultra-urban subwatersheds that lack surface area for stormwater treatment. The most common form of treatment is the underground sand filter which provides effective pollutant removal. Underground sand filters make sense when water quality and public health issues are paramount.

Ideal Conditions for Underground Retrofits

The most ideal situations for underground retrofits are in:

- Ultra-urban areas that lack available space on the surface for treatment
- Redevelopment or infill projects where stormwater treatment requirements are triggered
- Severe stormwater hotspots or central business districts
- Sites where untreated direct stormwater discharges to extremely sensitive waters (e.g., intake for drinking water supply, swimming beaches, harbors, shellfish beds, waterfronts)
- Sites where pretreatment is needed prior to another retrofit
- Regions that have underlying soils with exceptionally good infiltration rates (e.g., glacial till, outwash plains, sandy plains)
- Parking lots that cannot be served by a surface retrofit
- Public works yards where crews can perform frequent maintenance
- The receiving storm drain system is only a few feet below ground level
- Owner/operator is unwilling or unable to frequently maintain it

Restoration Alternatives in Ultra Urban Areas

It can be extremely expensive to retrofit ultra-urban subwatersheds using underground retrofits. Alternatives for improving stormwater quality in these subwatersheds include non-structural practices, such as:

- Intensive street sweeping (see CWP, 2008)
- Regular cleanouts of storm drain inlets (see CWP, 2008)
- Pollution prevention practices (see CWP, 2005a)
- Detection and elimination of illicit discharges (see Brown *et al.*, 2004)
- Municipal housekeeping practices (see CWP, 2008)

7.8. BASIC STEPS IN STORMWATER RETROFITTING

An eight step process is recommended to systematically search for retrofit storage in a subwatershed (**Figure 7.46** below). The process begins with retrofit scoping and concludes with maintenance of the constructed retrofit. Chapter 4 of the Center for Watershed Protection's *Manual 3: Urban Stormwater Retrofit Practices* (CWP, 2007a) provides more specific information on each step of the retrofit process. One key step in the process is conducting a Retrofit Reconnaissance Investigation to accomplish the following purposes:

- Verify the feasibility of candidate retrofit sites
- Collect information to create initial concept designs for retrofit projects
- Develop an organized and objective estimate of candidate sites' project costs and benefits

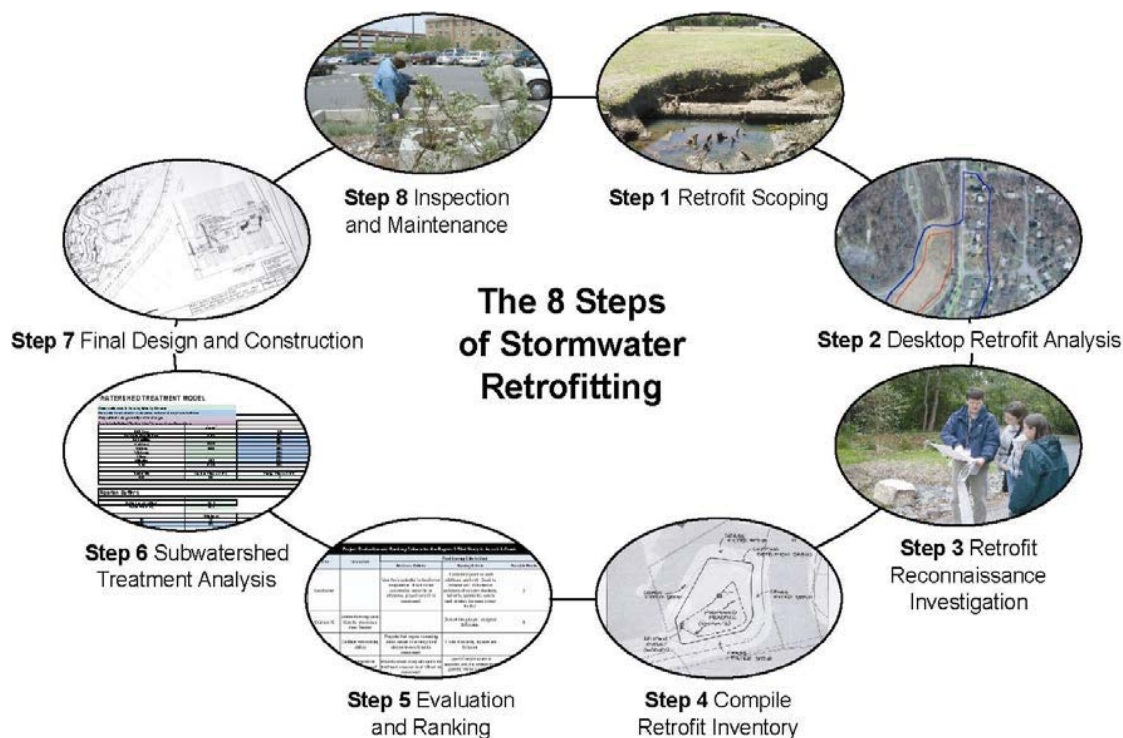


Figure 7.46. One Model of the Basic Steps of Stormwater Retrofitting
Source: CWP, 2007

7.8.1 Watershed Retrofit Inventory

The first two steps in the process, Retrofit Scoping and Desktop Retrofit Analysis, lead to development of an inventory of many potential sites where retrofit projects would be appropriate and feasible within the watershed or community. The best retrofit sites fit easily into the existing landscape, are located at or near major drainage or stormwater control facilities, and are easily accessible. Usually the first step is completed in the office using available topographic mapping, low altitude aerial photographs (where available), storm drain master plans, and land use maps (zoning or tax maps are generally acceptable). Many of these tools may be incorporated into a local GIS (**Figure 7.47** below)

Before venturing into the field, there are two tasks that should be performed. First, the drainage areas should be delineated, and second, the potential surface area of the facility measured. The drainage area is used to compute a capture ratio. This is the percentage of the overall watershed that is being managed by the retrofit project(s). The surface area is used to compute a preliminary storage volume of the proposed facility. These two bits of information can be used as a quick screening tool. In general, an effective retrofitting strategy should aim to capture at least 50% of the watershed. Ideally, the minimum target storage volume for each retrofit is approximately 0.5 inch per impervious acre.

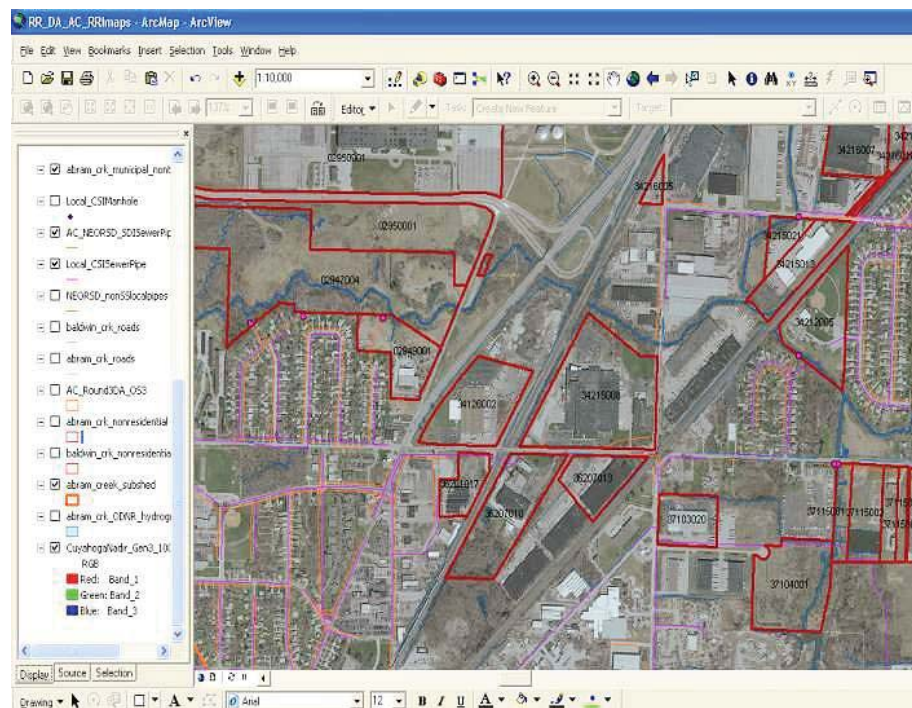


Figure 7.47. Desktop Analysis of Potential Retrofit Sites Using GIS System
(Source: CWP)

7.8.2 Field Verification of Candidate Sites



Figure 7.48. Retrofit Reconnaissance Investigation
(Source: CWP)

The next two steps involve a field reconnaissance and refinement of the site inventory. As shown in **Figure 7.48** above, the Retrofit Reconnaissance Investigation is one part of the process where local governments can involve volunteers from the community. **Appendix 7-B** provides a copy of the CWP's Retrofit Reconnaissance Investigation Checklist. Candidate retrofit sites identified during Scoping and Analysis should be investigated in the field to verify that they are indeed feasible sites. As well, the field exercise provides an opportunity to spot additional candidate sites that may not have been obvious from the resources analyzed earlier in the office. This field investigation involves a careful assessment of site specific information, such as:

- Presence of sensitive environmental features;
- Location of existing utilities;
- Type of adjacent land uses;
- Condition of receiving waters;
- Construction and maintenance access opportunities, and most importantly; and
- Evaluation of retrofit suitability.

Usually information is recorded on a Retrofit Reconnaissance Form (**Figure 7.49**), notes may be made on maps (**Figure 7.50**), conceptual sketches are prepared and photographs are taken. During field reconnaissance, utilities should be located and an assessment made as to potential conflicts that can raise costs. It is advisable to contact the appropriate utility to verify field observations and discuss the potential facility. This may alleviate potential conflicts later.

| Retrofit Reconnaissance Investigation | |
|---|---|
| Unique Site ID: <u>010</u> | Watershed: <u>RC</u> |
| City: <u>Albemarle County</u> | Subwatershed: <u>RC</u> |
| Date: <u>10-12-07</u> | Assessed By: <u>RC</u> |
| Site Description: | Picture: |
| Name: <u>Ludlow Run, Chesapeake Bay</u> | |
| Address or Nearest Intersection: <u>Chesapeake Bay, 2nd St, Chesapeake Bay</u> | |
| Parcel Number: <u>010-10-010</u> | |
| Ownership: <input checked="" type="checkbox"/> Public <input type="checkbox"/> Private <input type="checkbox"/> Unknown | |
| Potential Retrofit Location: | |
| <input checked="" type="checkbox"/> Existing Pond <input type="checkbox"/> Above Roadway Crossing <input type="checkbox"/> On Site | |
| <input type="checkbox"/> Below Outlet <input type="checkbox"/> In Conveyance System <input type="checkbox"/> Small Impervious Area | |
| <input type="checkbox"/> In Transit Pond <input type="checkbox"/> Large Parking Lot <input type="checkbox"/> Small Parking Lot | |
| <input type="checkbox"/> Other <input type="checkbox"/> Individual Street <input type="checkbox"/> Landscape / Hardscape | |
| <input type="checkbox"/> Other <input type="checkbox"/> Individual Roof <input type="checkbox"/> Underside | |
| Drainage Area to Site: | |
| Drainage Area: <u>0.1000 ac</u> | Drainage Area Land Use: |
| Imperviousness: <u>20%</u> | <input type="checkbox"/> Institutional |
| Impervious Area: <u>0.0200 ac</u> | <input type="checkbox"/> Industrial |
| Notes: <u>Area is a wetland</u> | <input type="checkbox"/> 50% (in 1 ac lots) |
| Drainage Area: <u>0.1000 ac</u> | <input type="checkbox"/> Transport Related |
| Drainage Area: <u>0.1000 ac</u> | <input type="checkbox"/> Park |
| Drainage Area: <u>0.1000 ac</u> | <input type="checkbox"/> Multi-Family |
| Drainage Area: <u>0.1000 ac</u> | <input type="checkbox"/> Undeveloped |
| Drainage Area: <u>0.1000 ac</u> | <input type="checkbox"/> Other |
| Existing Site Conditions: | |
| Existing Stormwater BMP: | |
| <input checked="" type="checkbox"/> Yes <input type="checkbox"/> No <input type="checkbox"/> Possible | |
| Describe Existing Site Conditions, including Existing Drainage and Conveyance Patterns: | |
| <p>Not certain about the structures purpose, is shaped like a dry detention basin, but is open where a small creek enters and exits it. Structure where once exists appears to have large fence (see sketch).</p> <p>The height of the existing basin edges varies. At the point where the creek enters the basin, it is about 10' high. At the point where the creek exits, the height is 8'. The peak of the south wall is 11' and tapers to 9'. The creek enters under the fence on the east side at 3'. The wall along the fence line peaks at 12' on the left side, and then tapers to 10' on the right side. Drainage from the basin is via a small pipe on the right side. The water remains on the north side. Creek flows along south side of basin.</p> | |

Figure 7.49. Retrofit Recon Form
Source: CWP



Figure 7.50. Field Notes on an Aerial Photo
Source: CWP

The sensitivity of existing natural resources, such as wetlands, streams, and forests, should be evaluated. Impacts to these resources should be avoided or minimized, if possible. Finally, adjacent land uses should be identified and evaluated for opportunities to install stormwater control measures that are compatible with nearby properties.

7.8.3. Prioritize Sites for Implementation (Figure 7.51)

Once sites have been identified and determined to be feasible and practical, the next step is to set up a plan for future implementation. It is prudent to have an implementation strategy based on a predetermined set of objectives. For example, in some watersheds, implementation may be based on a strategy of reducing pollutant loads to receiving waters where the priority of retrofitting might target the land uses or sites with the highest pollutant loadings first. Whereas if the strategy is oriented more towards restoring stream channel morphology, priority retrofits might be targeted to capture the largest drainage areas and provide the most storage.

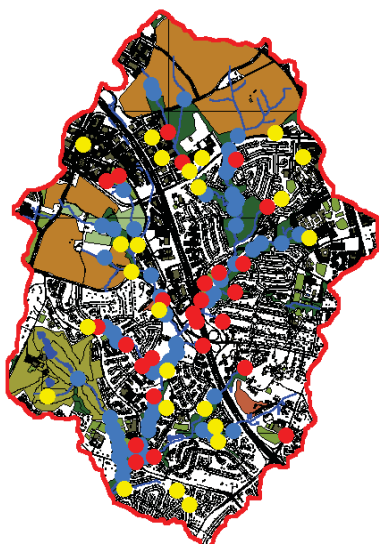


Figure 7.51. Retrofit Priorities Map
Source: CWP

| Category | Ranking Factor | Potential Score | |
|--|--|-----------------|------------|
| Potential for Treatment | | | 40 |
| | Total drainage area treated | 15 | |
| | Impervious area treated | 15 | |
| | Land use treated | 10 | |
| Potential for Greater Ecological and Community Benefits | | | 25 |
| | Potential education / demonstration project AND/OR public accessibility | 15 | |
| | Treatment of area(s) of concern | 5 | |
| | Potential to support other planned or on-going watershed restoration projects and activities | 5 | |
| Site Constraints | | | 35 |
| | Access for construction and maintenance | 15 | |
| | Potential for conflict with existing utilities | 5 | |
| | Potential ecological conflicts | 10 | |
| | Project for further consideration (yes/maybe) | 5 | |
| Total Potential Score | | | 100 |

Figure 7.52. Retrofit Priority Scoring Sheet
Source: CWP

Whatever the restoration focus, it is useful to provide a scoring system that can be used to rank each retrofit site based on a uniform criteria (**Figure 7.52**). A typical scoring system might include a score for the following items:

- Pollutant removal capability
- Stream channel protection capability
- Flood protection control capability
- Cost of facility (design, construction and maintenance costs)
- Ability to implement the project (land ownership, construction access, permits)
- Potential for public benefit (e.g., education, location within a priority watershed, visible amenity, supports other public involvement initiatives, etc.)

7.8.4 Public Involvement Process

This step in the process is not noted in the Center for Watershed Protection's model of the process, as shown in **Figure 7.46** above. However, this aspect of the process is critical in order to gain support for retrofit installations. A successful project must involve the immediate neighbors who will be affected by the changed conditions. Nearly all retrofits require modifications to the existing environment. A dry detention pond may be a very desirable area for some residents in the community. It is a community space and only rarely is there any water in the pond. A stormwater pond or wetland retrofit, on the other hand, may have large expanses of water and may have highly variable water fluctuations. Adjacent owners may resist these changes. In order to gain citizen acceptance of retrofits they must be involved in the process from the start and throughout the planning, design and implementation process. Citizens who are informed about the need for, and benefits of, retrofitting are more likely to accept projects.

Still, some citizens and citizen organizations will never support a particular project. This is why it is mandatory that there be an overall planning process which identifies potential projects early in the selection process and allows citizen input before costly field surveys and engineering designs are performed. Project sites and retrofit techniques that simply cannot satisfy citizen concerns may need to be dropped from further consideration.

A good retrofit program must also incorporate a good public relations plan. Slide shows or field trips to existing projects can be powerful persuasions to skeptical citizens. Every site that goes forward to final design and permitting should be presented at least once to the public through a public hearing or "town hall" type meeting.

7.8.5. Retrofit Design

This step involves the subwatershed or drainage treatment analysis (**Figure 7.53**) and detailed engineering designs and construction plans of the retrofit practices (**Figures 7.54 and 7.55**, below).

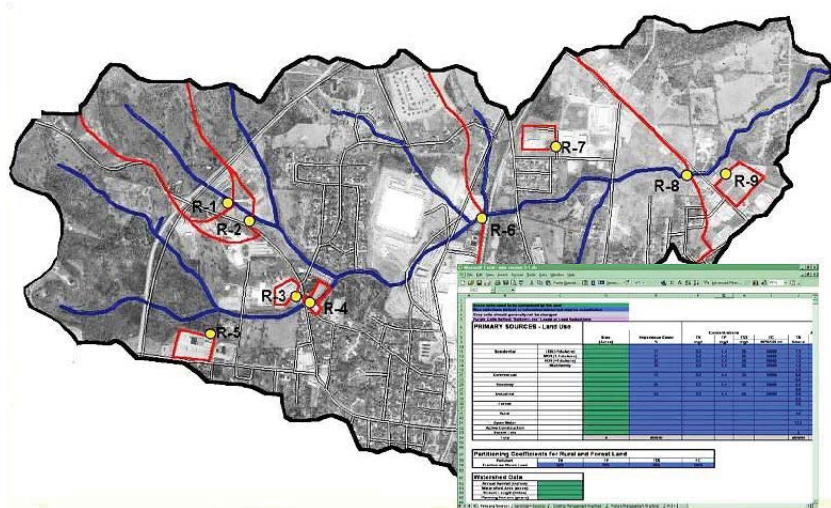


Figure 7.53. Subwatershed Analysis

Source: CWP



Figure 7.54. Conceptual Retrofit Design Layout for a School Site

Source: CWP

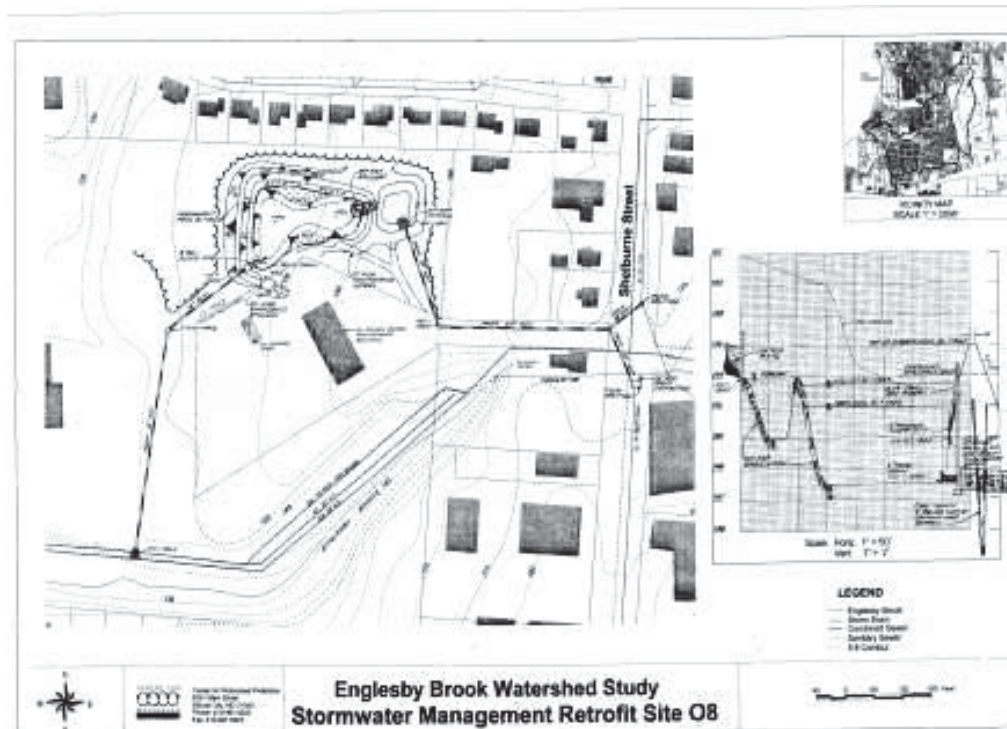


Figure 7.55. Engineering Drawing of a Retrofit Project

Source: CWP

Design of retrofit projects should incorporate the same elements as any other structural control design including, but not limited to the following:

- Adequate hydrologic and hydraulic modeling
- Detailed topographic mapping
- Property line establishment
- Site grading
- Structural design
- Geotechnical investigations
- Erosion and sediment control design
- Construction phasing and staging

Normal structural control design usually follows prescribed design criteria (e.g., control of the 1-year and 10-year storms, sizing for a specified treatment volume, etc.). Retrofit designers must work backwards from a set of existing site constraints to arrive at an acceptable level of stormwater control obtainable. Also, a preliminary cost estimate should be a part of the design phase. Retrofits can vary widely as to cost from a few thousand dollars to several hundred thousand dollars. This process may yield facilities that are too small or ineffective, or too costly for the benefit achieved, and therefore not practical for further consideration.

Designers should look for opportunities to combine projects, such as stream stabilization or habitat restoration with the retrofit in a complementary manner. For example, Green Street retrofits might be held until a target street must resurfaced or dug up for utility repair or replacement. The key to successful retrofit design is in balancing the ability to achieve the maximum pollutant removal, channel erosion protection and flood control while limiting the impacts to adjacent infrastructure, residents or other properties. Designers must consider issues like avoiding relocations of existing utilities, minimizing existing wetland and forest impacts, maintaining existing floodplain elevations, complying with dam safety and dam hazard classification criteria, avoiding maintenance nuisance situations, and providing adequate construction and maintenance access to the site.

7.8.6. Permitting, Construction, Inspections and Maintenance

Perhaps the most difficult permitting issues for retrofit projects involve impacts to wetlands, forests and floodplain alterations. Many of these impacts are either unavoidable or necessary to achieve reasonable storage targets. The primary issues that permitting agencies are looking for is to ensure that the impacts have been minimized to the maximum extent practicable and that the benefits of the proposed project are clearly recognizable and justifiable. In some instances, mitigation may also be required in order to satisfy permitting. If so, additional costs may be involved.

Like any design project, proper construction, inspection, and administration is integral to a successful facility. Retrofitting often involves construction of unique or unusual elements, such as flow splitters, underground sand filters, or stream diversions. Many of these practices may be unfamiliar to many contractors. Most publicly funded projects are awarded to the low bidder who may be qualified to do the work, but may never have constructed projects of this nature.

/Therefore, it is almost a necessity to retain the retrofit designer of record or other qualified professional to answer contractor's questions, approve shop drawings, conduct regular inspections, hold regular progress meetings, conduct construction testing, and maintain construction records. Preparation of As-Built drawings should also be a part of the construction process. These drawings are used for long-term maintenance purposes.

Always the last element and often the least practiced component of a stormwater management program, maintenance is doubly important in retrofit situations. The reasons are simple: most retrofits are undersized when compared to their new development counterparts, and space is at a premium in urban areas where provisions such as access roads and stockpiling or staging areas are either absent or woefully undersized.

7.9 CALCULATING NUTRIENT REDUCTIONS FOR INDIVIDUAL RETROFIT PROJECTS

Stormwater retrofits are a diverse group of BMP projects that provide nutrient and sediment reduction from existing development that is currently untreated by any BMP or is inadequately treated by an existing BMP. The amount of nutrient reduction that results from any particular stormwater retrofit BMP depends on several factors:

- The class of retrofit (e.g., new and full-sized, an existing BMP being enhanced, etc.);
- The specific type of retrofit BMP selected (e.g., disconnection, bioretention, etc.);
- The baseline nutrient load to the BMP
- The volume of rainfall captured
- The amount of runoff volume reduction achieved

Retrofits can be problematic when it comes to defining a nutrient removal rate. For example:

- Every retrofit project is unique to some degree, depending on the drainage area it treats, the treatment mechanism(s) of the selected BMP, the runoff volume it captures, and the degree of prior stormwater treatment at the site, if any. Due to site constraints, many retrofit BMPs are under-sized in comparison to new BMPs designed to new development standards. This typically results in reduced pollutant removal capacity. Some adjustment in pollutant removal capability is needed to account for situations where the retrofit BMP cannot capture and treat the specified treatment volume.
- There is virtually no research available specifically for stormwater retrofits, so removal rates must be inferred from other known BMP and runoff reduction performance data.
- Many retrofits employ innovative combinations of runoff treatment mechanisms and may not be easily classified according to the existing CBP- or state-approved BMP removal rates.
- Localities often evaluate dozens or even hundreds of candidate projects during retrofit investigations to find the best ones. Therefore, localities will need fairly simple protocols to estimate pollutant reduction achieved by individual retrofits projects as part of their watershed assessment and retrofit investigation.

7.9.1. Method for Calculating Retrofit BMP Nutrient Removal

Assigning a single universal pollution removal rate for stormwater retrofit BMP projects is not practical or scientifically defensible. Instead, DEQ proposes the use of a method developed by the USEPA Chesapeake Bay Program's Urban Stormwater Work Group (Schueler and Land, 2012), which has been endorsed by state SWM program managers throughout the Chesapeake Bay region. Using this method, the removal rate for each individual retrofit project is determined based on the amount of runoff it treats and the degree of runoff volume reduction it provides. For ease of use, a set of three curves was developed, as portrayed in **Figures 7.56 - 7.58**. The technical derivation of the curves is provided in **Appendix 7-C** of this chapter. There are four steps in this method:

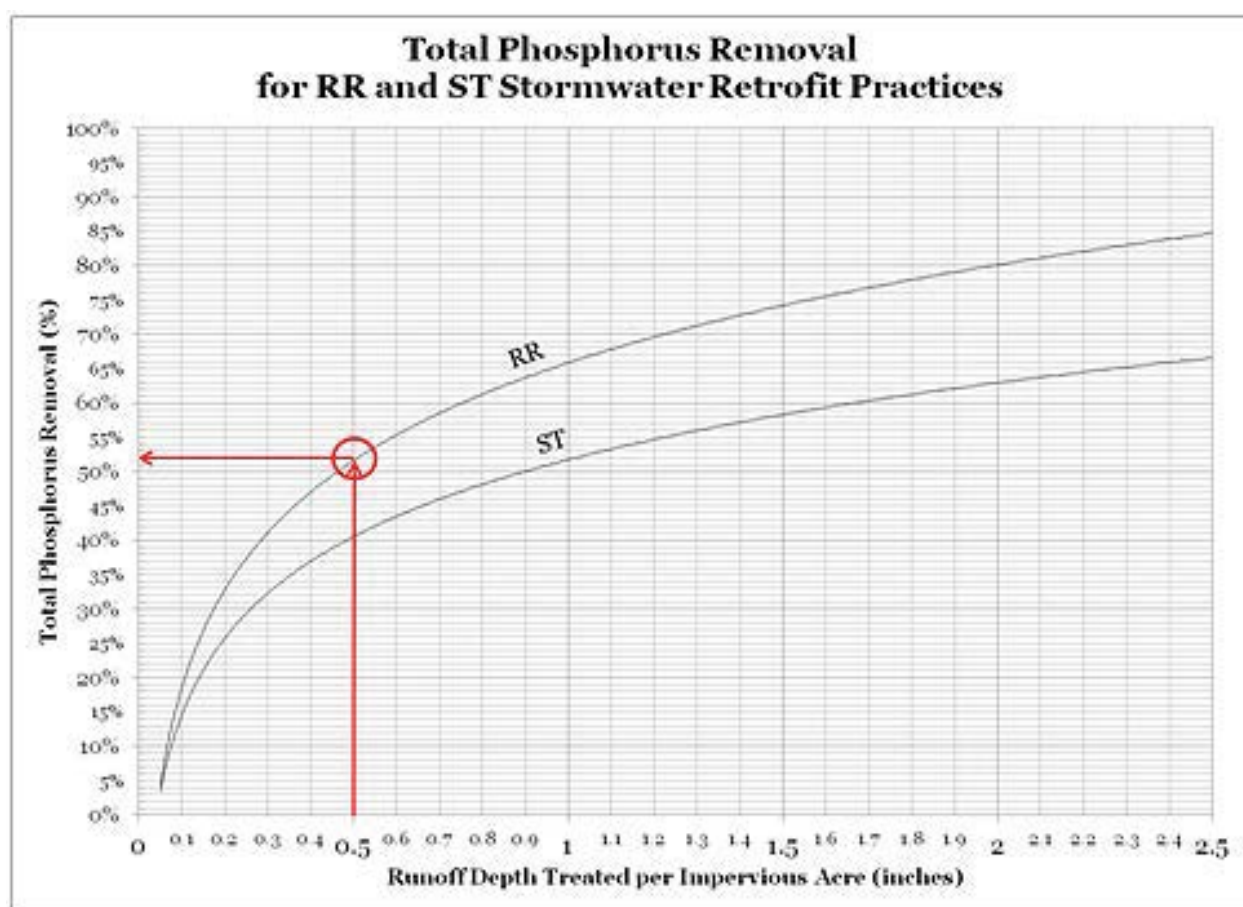


Figure 7.56. Retrofit Pollutant Removal Adjustor Curve for Total Phosphorus (TP)

- **Step 1:** Compute the baseline nutrient load (use TP as the indicator) for the site area draining to the proposed retrofit BMP using the Virginia Runoff Reduction Method spreadsheet. This calculation method closely tracks the EPA Chesapeake Bay Model projections for baseline nutrient loads for urban and suburban lands.

- **Step 2:** Select the appropriate method to define a project-specific retrofit removal rate, based on its appropriate retrofit classification:
 - New retrofit facility;
 - BMP conversion;
 - Existing BMP enhancement;
 - Greet Street retrofit; or
 - On-site LID retrofit
- **Step 3:** Adjust the removal rate using the runoff adjustment method.
- **Step 4:** Multiply the adjusted retrofit removal rate by the pre-retrofit baseline nutrient load to determine the total pounds of nutrients reduced by the retrofit project.

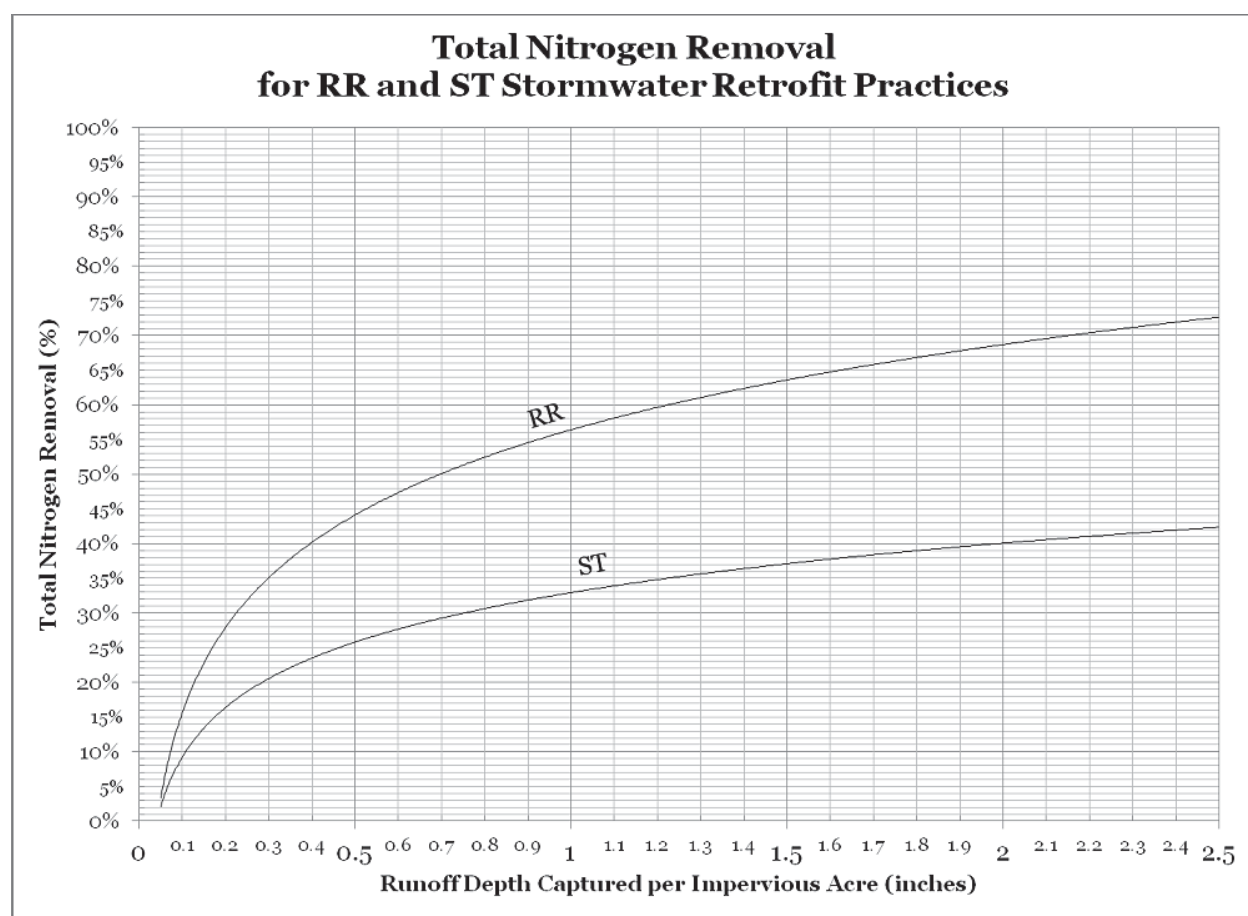


Figure 7.57. Retrofit Pollutant Removal Adjustor Curve for Total Nitrogen (TN)

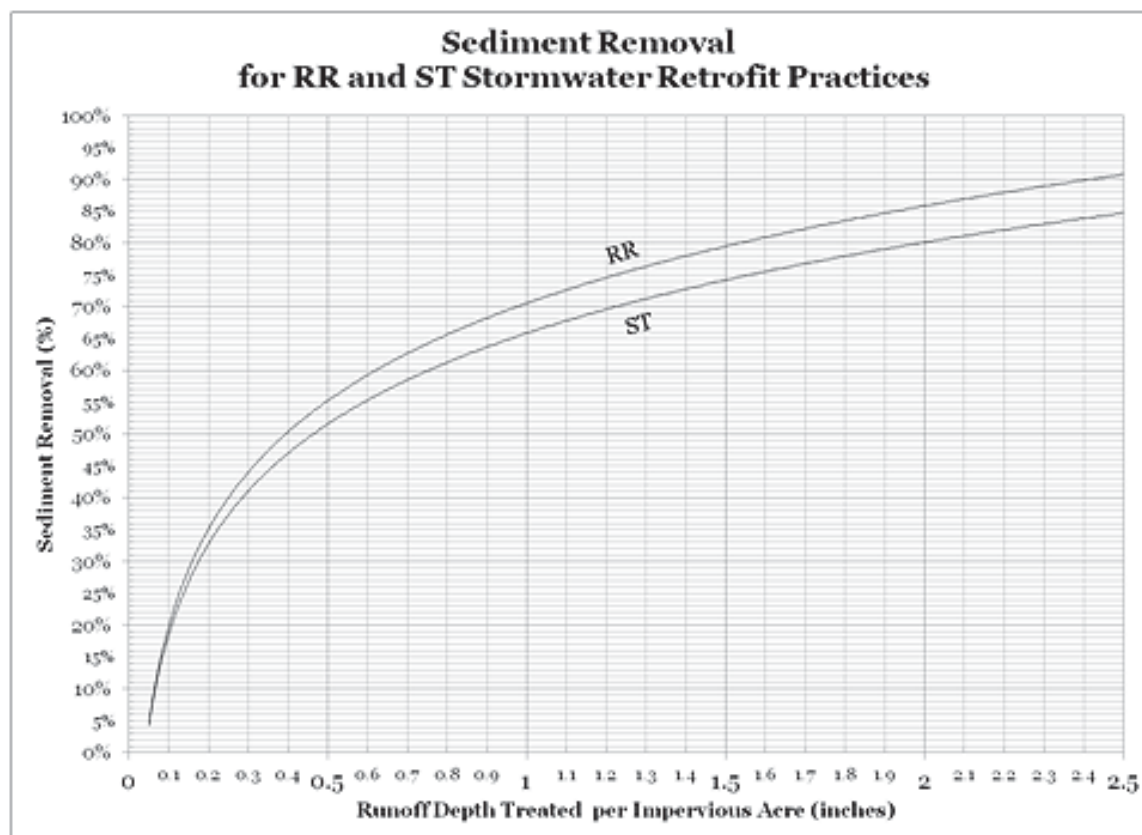


Figure 7.58. Retrofit Pollutant Removal Adjustor Curve for Total Suspended Solids (TSS)

In order to determine the runoff volume treated by a retrofit practice, the designer must first estimate the Runoff Storage volume (RS) in acre-feet. This, along with the Impervious Area (IA) in acres, is used in the standard retrofit equation to determine the amount of runoff volume in inches treated at the site:

$$= \frac{(RS)(12)}{IA}$$

Where:

RS = Runoff Storage Volume (acre-feet)

IA = Impervious Area (acres)

Once the amount of runoff captured by the practice is determined, the retrofit removal adjustor curves make it easy to determine pollutant removal rates for individual stormwater retrofits. The designer first defines the runoff depth treated by the project (on the x-axis), and then determines whether the project is classified as having runoff reduction (RR) or stormwater treatment (ST) capability (from **Table 7.7** below). The designer then goes upward to intersect with the appropriate curve, and moves to the left to find the corresponding removal rate on the y-axis. An example is provided in **Figure 7.56** above, using the TP removal adjustor curve (since TP is the basis for water quality compliance in the Virginia SWM Regulations). Removals for TN and TSS can be determined in like manner by using the adjustor curves in **Figure 7.57** and **Figure 7.58** above, respectively.

Table 7.7. Classification of BMPs based on Runoff Reduction Capability

| Runoff Reduction Practices (RR) | Stormwater Treatment Practices (ST) |
|--|---|
| Site Design/Non-Structural Practices | Constructed Practices |
| Landscape Restoration/Reforestation | Constructed Wetlands |
| Riparian Buffer Restoration | Dry Extended Detention Ponds |
| Rooftop Disconnection (aka Simple Disconnection to Amended Soils, to a Conservation Area, to a Pervious Area, Non-Rooftop Disconnection) | Filtering Practices (aka Constructed Filters, Sand Filters, Stormwater Filtering Systems) |
| Sheetflow to Filter/Open Space* (aka Sheetflow to Conservation Area, Vegetated Filter Strip) | Proprietary Practices (aka Manufactured BMPs) |
| All Environmental Site Design BMPS – Chapter 6 of the this Handbook | Wet Ponds (aka Retention Basin) |
| Constructed Practices | Wet Swale |
| Bioretention or Rain Garden (Standard or Enhanced) | |
| Dry Swale | |
| Expanded Tree Pits | |
| Grass Channels (w/ Soil Amendments, aka Bio-swale, Vegetated Swale) | |
| Green Roof (aka Vegetated Roof) | |
| Green Streets | |
| Infiltration (aka Infiltration Basin, Infiltration Bed, Infiltration Trench, Dry Well/Seepage Pit, Landscape Infiltration) | |
| Permeable Pavement (aka Porous Pavement) | |
| Rainwater Harvesting (aka Capture and Re-use) | |
| *May include a berm or a level spreader | |

Runoff reduction is defined as the total post development runoff volume that is reduced through canopy interception, soil amendments, evaporation, rainfall harvesting, engineered infiltration, extended filtration or evapotranspiration. Retrofit projects that achieve at least a 25% reduction of the annual runoff volume are classified as providing Runoff Reduction (RR), and therefore earn a higher net removal rate. Retrofit projects that employ a permanent pool, constructed wetlands or sand filters have less runoff reduction capability, and their removal rate is determined using the Stormwater Treatment (ST) curve.

Table 7.7 above assigns BMPs referenced in Bay State stormwater management manuals into either the ST or RR category, so that designers can quickly determine which curve they should use based on the primary treatment practice employed by the retrofit. In situations where a mix of ST and RR practices are used within the same retrofit project, the designer should use the curve based on either the largest single practice used in the project or the ones that provide the majority of the retrofit treatment volume.

The removal rates determined from the retrofit removal rate adjustor curves are applied to the *entire* drainage area to the retrofit, and not just its impervious acres. Also, the retrofit reporting unit is the *entire* treated area, regardless of whether it is pervious or impervious.

7.9.1.1. New Retrofit Facilities

This category includes new retrofit projects that create storage to reduce nutrients from existing developed land that is not currently receiving any stormwater treatment. Common examples of new retrofits include creating new storage upstream of roadway crossings, near existing stormwater outfalls, within the existing stormwater conveyance system or adjacent to large parking lots. Desktop and field methods for discovering opportunities for new retrofits are described in Schueler (2007). There are two options to define removal rates for this class of retrofit projects:

- **Chesapeake Bay Program (CBP) Rate Option:** If the new retrofit project can be classified into one of the existing CBP urban BMP categories and has enough treatment volume to treat the runoff from at least one inch of rainfall, then the appropriate CBP approved rates should be used (**Table 7.8** below).

Table 7.8. Current BMP Efficiencies Approved by the Chesapeake Bay Program (2/9/2011) ^{1, 2, 3}

| URBAN BMP | | MASS LOAD REDUCTIONS | | |
|---|---------------|----------------------|------------------|-----|
| | | Total Nitrogen | Total Phosphorus | TSS |
| Wet Ponds & Constructed Wetlands | | 20 | 45 | 60 |
| Dry Detention Ponds | | 5 | 10 | 10 |
| Dry Extended Detention Ponds | | 20 | 20 | 60 |
| Infiltration | | 80 (85) ⁴ | 85 | 95 |
| Filtering Practices (sand filters) | | 40 | 60 | 80 |
| Bioretention | C&D w/ UD | 25 | 45 | 55 |
| | A&B w/ UD | 70 | 75 | 80 |
| | A&B w/o UD | 80 | 85 | 90 |
| Permeable Pavement | C&D w/ UD | 10 (20) | 20 | 55 |
| | A&B w/ UD | 45 (50) | 50 | 70 |
| | A&B w/o UD | 75 (80) | 80 | 85 |
| Grass Channels | C&D w/o UD | 10 | 10 | 50 |
| | A&B w/o UD | 45 | 45 | 70 |
| Bioswale | aka Dry Swale | 70 | 75 | 80 |
| Nutrient Management | | 17 | 22 | NA |
| Street Sweeping | Bi-monthly | 3 | 3 | 9 |
| Forest Buffers | | 25 | 50 | 50 |

¹ In many cases, removal rates have been discounted from published rates to account for poor design, maintenance and age, and apply to generally practices built prior to 2008

² Current Practices are designed to more stringent design and volumetric criteria, and may achieve higher rates – see **Table 7.8** below

³ Some practices, such as forest conservation, impervious cover reduction, tree planting are modeled as a land use change. Urban stream restoration is modeled based on a reduction per linear foot of qualifying stream restoration project

⁴ Numbers in parentheses reflect design variation with a stone sump to improve long term infiltration rates

Table 7.9. Comparative Runoff Reduction and Nutrient Removal for Practices

| Practice | BMP Design Level | Runoff Reduction | TN EMC Removal ³ | TN Mass Load Removal | TP EMC Removal | TP Mass Load Removal ⁶ |
|--|--|-------------------------|-----------------------------|-------------------------|----------------------|-----------------------------------|
| Rooftop Disconnect | 1 ² | 25 to 50 ¹ | 0 | 25 to 50 ¹ | 0 | 25 to 50 ¹ |
| | No Level 2 Design | | | | | |
| Sheet Flow to Veg. Filter or Conserv. Open Space | 1 | 50 | 0 | 50 | 0 | 50 |
| | 2 ⁵ | 50 to 75 ¹ | 0 | 50 to 75 ¹ | 0 | 50 to 75 ¹ |
| Grass Channels | 1 | 10 to 20 ¹ | 20 | 28 to 44 ¹ | 15 | 24 to 41 ¹ |
| | No Level 2 Design | | | | | |
| Soil Compost Amendment | Can be used to Decrease Runoff Coefficient for Turf Cover at Site. See the design specs for Rooftop Disconnection, Sheet Flow to Vegetated Filter or Conserved Open Space, and Grass Channel | | | | | |
| Vegetated Roof | 1 | 45 | 0 | 45 | 0 | 45 |
| | 2 | 60 | 0 | 60 | 0 | 60 |
| Rainwater Harvesting | 1 | Up to 90 ^{3,5} | 0 | Up to 90 ^{3,5} | 0 | Up to 90 ^{3,5} |
| | No Level 2 Design | | | | | |
| Permeable Pavement | 1 | 45 | 25 | 59 | 25 | 59 |
| | 2 | 75 | 25 | 81 | 25 | 81 |
| Infiltration Practices | 1 | 50 | 15 | 57 | 25 | 63 |
| | 2 | 90 | 15 | 92 | 25 | 93 |
| Bioretention Practices | 1 | 40 | 40 | 64 | 25 | 55 |
| | 2 | 80 | 60 | 90 | 50 | 90 |
| Urban Bioretention | 1 | 40 | 40 | 64 | 25 | 55 |
| | No Level 2 Design | | | | | |
| Dry Swales | 1 | 40 | 25 | 55 | 20 | 52 |
| | 2 | 60 | 35 | 74 | 40 | 76 |
| Wet Swales | 1 | 0 | 25 | 25 | 20 | 20 |
| | 2 | 0 | 35 | 35 | 40 | 40 |
| Filtering Practices | 1 | 0 | 30 | 30 | 60 | 60 |
| | 2 | 0 | 45 | 45 | 65 | 65 |
| Constructed Wetlands | 1 | 0 | 25 | 25 | 50 | 50 |
| | 2 | 0 | 55 | 55 | 75 | 75 |
| Wet Ponds | 1 | 0 | 30 (20) ⁴ | 30 (20) ⁴ | 50 (45) ⁴ | 50 (45) ⁴ |
| | 2 | 0 | 40 (30) ⁴ | 40 (30) ⁴ | 75 (65) ⁴ | 75 (65) ⁴ |
| Ext. Det. Ponds | 1 | 0 | 10 | 10 | 15 | 15 |
| | 2 | 15 | 10 | 24 | 15 | 31 |

Notes ¹ Lower rate is for HSG soils C and D, Higher rate is for HSG soils A and B.

² The removal can be increased to 50% for C and D soils by adding soil compost amendments, and may be higher yet if combined with secondary runoff reduction practices.

³ Credit up to 90% is possible if all water from storms of 1-inch or less is used through demand, and the tank is sized such that no overflow occurs. The total credit may not exceed 90%.

⁴ Lower nutrient removal in parentheses apply to wet ponds in coastal plain terrain.

⁵ See BMP design specification for an explanation of how additional pollutant removal can be achieved.

⁶ Total mass load removed is the product of annual runoff reduction rate and change in nutrient EMC.

- **Stormwater Retrofit Removal Rate Adjustor:** To determine the nutrient and sediment removal rates for an individual new retrofit project, the designer should use the appropriate adjustor curve (**Figures 7.56 - 7.58** above) to find the unique rate for the combination of runoff depth captured and runoff reduction/stormwater treatment that is achieved. The designer should also estimate the total contributing drainage area to the retrofit.

7.9.1.2. BMP Conversions

The specific method for defining the removal rate depends on the type and age of the BMP being converted:

- ***If the BMP being converted is a dry detention pond or flood control structure that currently is providing no effective water quality treatment***, then the existing BMP will have a zero removal rate. A higher CBP-approved BMP rate that reflects the improved stormwater treatment mechanism associated with the conversion can be taken directly from Table A-5 of Appendix A (i.e., dry ED, wet pond, constructed wetland or bioretention)
- ***If the BMP being converted involves a significant increase in runoff capture volume and/or an increase in runoff reduction, then an incremental rate is used.*** The removal rate for the existing BMP should be determined from the adjustor curve. A higher removal for the converted BMP will reflect the higher degree of runoff treatment and/or runoff reduction associated with the retrofit, as determined from the retrofit removal adjustor curves (**Figures 7.56 - 7.58** above). This method will generally be the most applicable to the majority of conversion retrofits.

In all cases, the designer should also estimate the total contributing drainage area to the retrofit. Examples are provided in the next section, that illustrate how both of these methods are applied to conversion retrofits.

7.9.1.3. Existing BMP Enhancements

This retrofit category applies to projects whereby the basic treatment mechanism of the existing BMP is not changed, but its nutrient reduction capability is enhanced by increasing its treatment volume and/or increasing the hydraulic retention time within the practice. BMP enhancements are a good strategy for older and larger ponds and wetlands built under less stringent sizing and design standards. BMP enhancement may also be a good strategy for the first generation of bioretention and filtering practices, which had design criteria that lacked the features now known to enhance nutrient removal.

An example of a retrofit enhancement for an older wet pond might be to increase its treatment volume, realign inlets to prevent short circuiting, add internal cells and forebays to increase flow path, and add aquatic benches, wetland elements and possibly even floating islands to enhance overall nutrient reduction.

At first glance, it would seem to be difficult to assign removal rates for such BMP enhancements, although Virginia now uses a two-level design system shared by many Bay states, whereby nutrient removal rates are increased when certain treatment volume and design features are met or exceeded (<http://www.vwrrc.vt.edu/swc/NonProprietaryBMPs.html>). Therefore, the recommended option to estimate the nutrient reduction achieved by BMP enhancement retrofits is as follows:

- **Step 1:** The base nutrient removal rate for the existing BMP (prior to enhancement) should be rate originally assigned to the BMP design under the Virginia Chesapeake Bay Preservation Act Program or the 1999 Virginia Stormwater Management Handbook. If the BMP is older than either of these programs (i.e., pre-1990), use the conservative CBP-approved rates found in **Table 7.8** above.
- **Step 2:** The designer should then evaluate the range of BMP enhancements to see if they qualify for the higher Level 1 or Level 2 rates shown in **Table 7.9** above.
- **Step 3:** The nutrient removal rate for the retrofit is then computed as the difference from the Level 1 or 2 rates and the existing Virginia or CBP-approved rate.

As an alternative, the nutrient and sediment removal rates for individual BMP enhancement retrofits are also expressed as an incremental removal rate (enhanced BMP - existing BMP).

- The rate for the existing BMP is defined based on its combination of runoff treatment and runoff reduction using the retrofit removal adjustor curves (**Figures 7.56 - 7.58** above). Designers may reduce the actual amount of runoff treatment in the existing BMP that is not effective (e.g., treatment volume that is ineffective because of short-circuiting or other design problems that reduce the hydraulic retention time).
- The enhanced BMP will have either a greater runoff treatment volume and/or achieve a better runoff reduction rate. Designers can determine the higher rate for the enhanced BMP using the retrofit removal adjustor curves.
- The removal rate for the BMP enhancement is then defined as the difference between the enhanced rate and the existing rate. An example of how to apply this protocol for BMP enhancements is provided in the next section.

7.9.1.4. BMP Restoration

The removal rate for BMP restoration depends on whether the existing BMP has been previously reported and included in the state's CBWM input deck.

- If the BMP was installed prior to July 1, 2009 and has *not* been previously reported, then the BMP is considered to be a new retrofit facility with the applicable removal rate from (1) the 1999 edition of the *Virginia Stormwater Management Handbook*, if applicable, (2) **Table 7.8** above, if a CBP-approved BMP, or (3) the as determined by using the retrofit removal adjustor curves for the drainage area contributing to the BMP.

- If the BMP was installed prior to July 1, 2009 and *is* included in the state's CBWM input deck, then the removal rate for a restored BMP is expressed as an incremental removal rate (restored BMP - existing BMP). The existing BMP removal rate is determined from (1) the 1999 edition of the *Virginia Stormwater Management Handbook*, if applicable, (2) **Table 7.8** above, if a CBP-approved BMP, or (3) using the curves for all others, based on the original BMP sizing and design criteria. The restored BMP rate is defined using the retrofit removal rate adjustor curve for the runoff treatment volume "restored" (i.e., by sediment cleanouts, vegetative harvesting or practice rehabilitation) and/or shifting to RR runoff reduction (i.e., media replacement).

To prevent double counting in reports to the USEPA Chesapeake Bay Program (for TMDL accounting purposes), the removal rate of a restored BMP should be reported to EPA in a two-step process. First, it should be reported at the degraded condition (lower/original removal rate) for at least one annual model progress run. Second, the incremental improvement associated with the BMP's restoration should then be reported for the next year's model progress run.

7.9.1.5. Green Street Retrofits

Green Streets use a combination of LID practices within public street rights-of-way, and they are gaining popularity as an attractive option to treat stormwater runoff in highly urban watersheds (CSN, 2011c). Green Streets provide many urban design benefits and create a more attractive and functional urban streetscape. Green Streets typically involve a combination of practices such as permeable pavers, street bioretention, expanded tree pits, individual street trees, impervious cover removal, curb extensions and filtering practices. The linear nature of Green Streets makes them a very efficient composite LID practice that can treat several acres of impervious cover in a single system.

Numerous Green Street or Green Alley demonstration projects have been installed in cities within the Bay watershed. At the current time, there is no standard design for Green Streets, since each project must deal with unique constraints present in each individual Green Street section (e.g. street width, right-of-way width, underground utilities, development intensity, parking needs, street lighting, and pedestrian/automotive safety, etc.).

Consequently, it is not feasible to assign a generic nutrient and sediment removal rate for Green Streets at this time. As an alternative, the nutrient removal credit for Green Streets can be estimated in a simple two-step process:

- **Step 1: Impervious Cover Reduction Credit.** The Simple Method (Schueler, 1987) can be used to compute the change in nutrient load that can be attributed the reduction in impervious cover associated with a narrower street. This is easily done by adjusting the site runoff coefficients to reflect the lower impervious cover associated with the Green Street.
- **Step 2.** The Green Street project can then be analyzed as a whole to determine the actual rainfall depth it controls and degree of runoff reduction it achieves. Based on these factors, designers can select the appropriate mass removal rates from the retrofit removal adjustor curves (**Figures 7.56 - 7.58** above).

7.9.1.6. On-Site LID Retrofits

This category includes the installation of a large number of small on-site retrofits, such as rain gardens, compost amendments, rain barrels, rooftop disconnections and tree planting, at the scale of a residential neighborhood. These retrofits are typically delivered by local governments, watershed groups, or neighborhood associations, who provide incentives and subsidies to individual property owners to implement them. In many cases, dozens or even hundreds of these small retrofits might be installed in any given subwatershed.

To simplify analysis, it is recommended that localities record the cumulative area of impervious cover treated by on-site retrofits, based on the average rainfall depth that is controlled, designers can select the appropriate mass removal rates from the retrofit removal adjutor curves (**Figures 7.56 - 7.58** above).

7.9.1.7. Local Tracking, Reporting and Verification

Localities should maintain a project file for each retrofit project installed. The file should be maintained for the entire period of time during which the retrofit nutrient removal credit will be claimed. The typical duration for the credit will be approximately 25 years, although the locality may be required to conduct a performance inspection at least once every five years to verify that the practice is being adequately maintained and operating as designed.

Localities should also submit some basic documentation to the state about each retrofit, including the following:

- GPS coordinates for the project location
- The year the retrofit project was installed
- Identify the type of BMP (BMP name)
- Identify the class of BMP (e.g., new retrofit facility or existing BMP retrofit as converted, enhanced, restored, etc.)
- The 12-digit watershed (Virginia 6th Order Watershed) in which it is located
- The total drainage area and impervious cover area treated
- The runoff volume treated or reduced (optional)
- The nutrient (and sediment) reduction credits claimed (and the method used to compute them)
- A signed certification that the retrofit has been inspected after construction and meets its performance criteria.

Localities are also responsible for long-term inspection and maintenance of permanent SWM BMPs and will be held accountable for the level of their continued performance, if they are located within the Virginia portion of the Chesapeake Bay watershed and subject to Chesapeake Bay TMDL pollution reduction targets. Localities are encouraged to develop a GIS-based BMP tracking system in order to schedule routine inspections and maintenance activities over time.

7.9.1.8. The Baseline Load Issue

This method for calculating pollutant removal loads for retrofit BMP projects does not require localities to define a pre-retrofit baseline load. However, DEQ acknowledges that many localities may want to estimate pre-retrofit baseline loads when it comes to finding the most cost-effective combination of retrofit projects to pursue in their local subwatershed retrofit investigations. Consequently, the Department recommends several options to use in planning level analyses of comparative retrofit load removal capability. These include the:

1. Generic state-wide CBWM urban unit loading rates
2. Simple Method (Schueler, 1987)
3. Watershed Treatment Model

Baseline loads are not needed to justify retrofit load reductions over time in the context of the Chesapeake Bay TMDL, since the CBWM calculates these directly based on the model segment in which the retrofit is located.

7.9.1.9 Analyzing Retrofit Options in the Context of CAST/VAST

A retrofit assessment protocol may not fit easily within the context of assessment and scenario builder tools, such as Chesapeake Assessment Scenario Tool (CAST) and Virginia Assessment Scenario Tool (VAST), that have been recently developed to assist states and localities to evaluate BMP options to develop watershed implementation plans (i.e., each retrofit has a unique rate and consequent load reduction, while the CAST and VAST apply a universal rate for all retrofits).

The USEPA Chesapeake Bay Program's modeling team has expressed a willingness to incorporate the adjustor curves presented above into the CAST modeling framework in the next year or so. Until such refinements are made, it is reasonable, for planning purposes, to assign a single removal rate to characterize the performance of a generic type of retrofit in order to evaluate alternative BMP scenarios. For example, DEQ could assume a generic stormwater retrofit that is a 50/50 blend of runoff reduction (RR) and stormwater treatment (ST) and treat 1-inch of runoff from the impervious area. This generic retrofit rate could be used in the context of CAST/VAST to compare load reductions for different levels of local drainage area treated by retrofits.

7.10. RETROFIT ECONOMICS

The first generation of retrofits primarily focused on demonstrating that retrofits could achieve restoration objectives, with little attention devoted to finding the least costly restoration solution. The next generation of retrofits, however, will need to demonstrate that they represent the most cost effective solution to the restoration problem they are designed to address. Some key findings on retrofit economics from the 2006 cost survey are shared below.

Retrofitting can be a costly enterprise. The cost to construct retrofits ranges from 1.5 to 4 times greater than the cost to construct stormwater practices at new development sites. The extra costs for retrofits are related to site constraints, higher excavation costs, greater design complexity,

more construction contingencies, additional engineering studies, enhanced landscaping and the experimental nature of many designs. Given that many retrofits are prototypes, it is expected that unit costs may decline in the future as contractors gain more familiarity with them.

There may be rare instances when retrofit costs can be based on new practice cost equations, but only when: land is abundant to provide maximum flexibility in site layout, site topography is such that a neutral earth balance can be attained and no major investments are contemplated for special plumbing, environmental permits, utility relocation or major landscaping.

Figure 7.59 below compares the median and quartile range in base construction cost for 18 different retrofit techniques. As can be seen, pond retrofits, rain gardens and new storage retrofits are the least expensive to construct, whereas ultra-urban techniques such as underground filters, tree pits, permeable pavers and green rooftops are the most expensive. The design team should carefully review these unit costs during initial scoping to ensure they are targeting the most cost-effective retrofits in a subwatershed.

Storage retrofits are generally more cost-effective than on-site retrofits, primarily due to economies of scale related to the large drainage areas they treat. In general, retrofits serving the smallest drainage areas tend to have the greatest unit cost. This finding suggests that designers should try to exhaust all possibilities for storage retrofits in a subwatershed before they embark on an onsite retrofit approach.

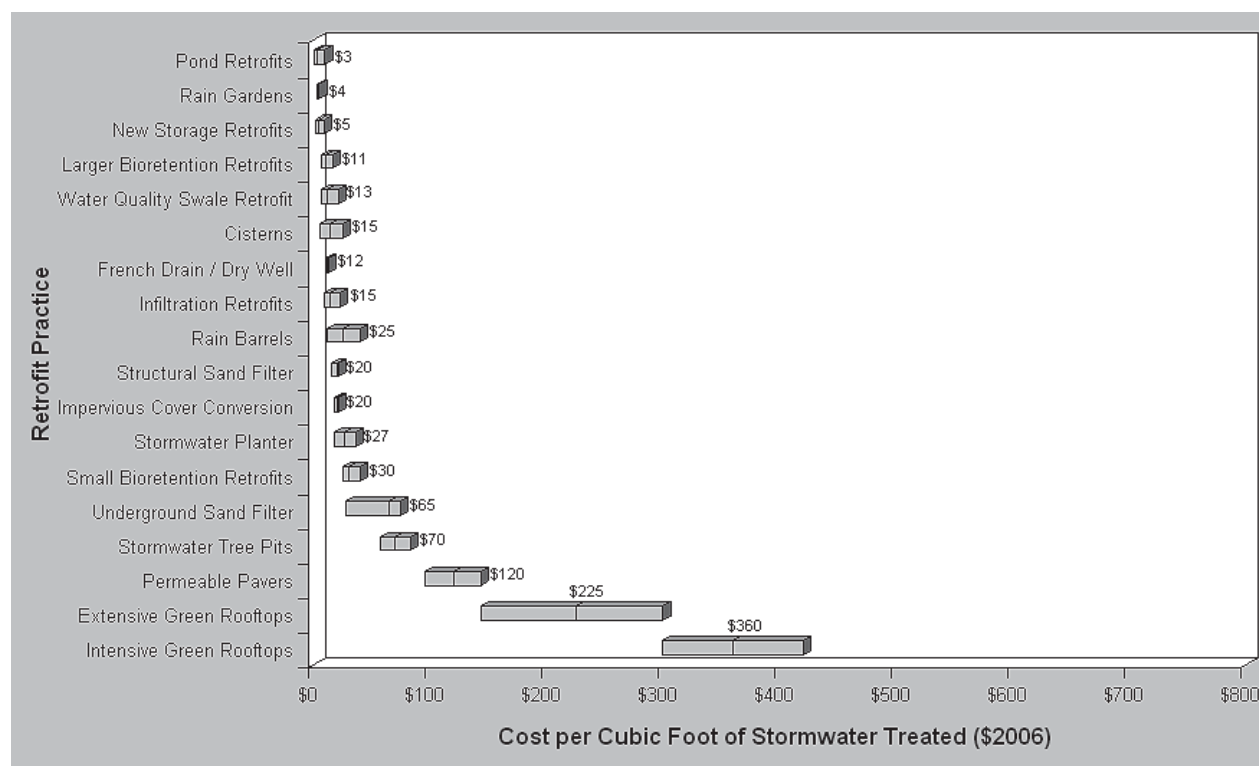


Figure 7.59. Range of base construction costs for various retrofit options.

(Note: Boxes show 25% and 75% quartiles; the line represents the median)

Source: CWP 2007

Construction costs for the same retrofit technique can vary by two orders of magnitude. For example, the unit construction cost for the least and most expensive pond retrofits ranged from \$1,350 to \$107,000 per impervious acre treated. An even wider range was reported for bioretention retrofits (\$2,000 to \$327,000 per impervious acre). Designers should always look for key factors that can drive up the cost of retrofitting when they evaluate individual retrofit sites.

The design and engineering (D&E) costs for both on-site and storage retrofits ranges from 32 to 40% of base construction cost (higher end when environmental permits must be secured). Total D&E costs for retrofits are higher than new stormwater practices, given their higher base construction costs. Land acquisition costs for all storage retrofits are assumed to be zero since they are generally constructed on public land. However, land acquisition costs must be added if land rights or easement need to be secured to build a project. On-site retrofits also have a hidden cost to persuade owners to install them on private land. The program cost to promote and deliver on-site retrofits may rival actual construction costs. Lastly, the retrofit costs shown here do not include the cost to find, assess and rank retrofits at the subwatershed level.

The most important number is the aggregate cost to construct retrofits across an entire subwatershed. Returning to the 5,000 acre subwatershed example, assume that 70% retrofit coverage is desired. If it is further assumed that storage retrofits are used to obtain 80% of the subwatershed treatment and on-sites for the remainder, it is possible to get a sense of the number and cost of retrofits needed for the subwatershed (**Table 7.10** below). At 10% subwatershed impervious cover, the retrofit bill is nearly \$7 million and climbs to \$20 to 40 million at higher levels of subwatershed impervious cover. While most communities spread out this investment over 5 or 10 years, it clearly underscores the need to devise creative retrofit delivery strategies to get the job done.

Table 7.10. Long Term Costs to Retrofit a 5,000 Acre Subwatershed

| Subwatershed Impervious Cover | Impervious Acres Treated | Number of Retrofits Required | Base Construction Costs | Total Restoration Cost |
|---|--------------------------|------------------------------|-----------------------------|------------------------|
| 10% | 353 | OS = 141 SR = 6 | \$1,582,000 \$3,579,000 | \$6,700,000 |
| 30% | 1,088 | OS = 435 SR = 17 | \$4,892,500 \$10,965,000 | \$20,600,000 |
| 45% | 1,650 | OS = 660 SR = 26 | \$7,425,000 \$16,740,000 | \$31,400,500 |
| 60% | 2,194 | OS = 878 SR = 35 | \$9,900,000 \$22,000,000 | \$41,500,000 |
| Assumptions: <ul style="list-style-type: none"> • 50 acres treated per storage retrofits (SR) and 0.5 acre treated per on-site (OS) retrofit • 70% of the entire subwatershed area to be retrofit • 80% of the watershed is treated by storage retrofits; 20% is treat with on-site retrofits • Storage retrofits are equally split between pond retrofits and new facilities • 25% of on-site retrofits are on residential land and 75% are on non-residential sites • Costs per impervious acre treated are: \$9,500 for pond retrofits; \$15,500 for new storage facilities; \$15,000 for residential on-site retrofits; and \$25,000 for non-residential on-site retrofits • Total cost includes D&E at 32% of base construction cost | | | | |

Source: CWP, 2007

Also keep in mind that costs are likely to be higher at the demonstration stage, but they will come down when all involved, from public officials to designers and contractors, have more experience and confidence with the process. For example, the first green street retrofit projects in a community can be very costly and time-consuming, especially if multiple municipal agencies (streets, utilities, zoning, etc.) must independently permit the project. However, permitting can be streamlined with experience.

7.10.1. Tips for Cost-Effective Retrofit Implementation

Localities can take the following steps to maximize the cost-effectiveness of BMP retrofits implemented within their jurisdictions.

- ***Maximize the Use of Other Nutrient Reduction Practices.*** Implementing source control and “good housekeeping” practices, such as those in the following list, throughout the community can boost pollution reduction efforts and reduce the number of retrofits needed to reach local pollution reduction targets.
 - Reforestation
 - Stream restoration
 - Fertilizer restrictions
 - Septic system upgrades
 - BMP maintenance upgrades
 - Stream buffer upgrades
 - Providing appropriate incentives and credits for redevelopment
 - Street sweeping
 - Elimination of illicit discharge
- ***Develop Multiple Revenue Streams and Retrofit Delivery Mechanisms*** (Figure 7.60 below), such as the following:
 - Stormwater Utility credits or discounts (as incentives)
 - Capital Improvement Budget (funding source)
 - Stormwater maintenance budgets (funding source)
 - Stormwater offset fees (funding source)
 - Nutrient trading (funding source and/or delivery mechanism)
 - Public-private partnerships (funding source and/or delivery mechanism)
 - BMP Maintenance enforcement (delivery mechanism)
 - Piggyback on street/utility reconstruction (delivery mechanism)
 - Piggyback on municipal construction projects (delivery mechanism)
- ***Maximize the Drainage Area Treated by Individual Retrofits.*** Large storage-type retrofits are usually the most cost-effective solutions, although they do require more permitting, easements and neighborhood consultation. Experience has shown that storage retrofits can treat up to 20% – 30% of the subwatershed area in suburban areas (much less in dense urban areas). After storage retrofits, Green Street and LID-type retrofits are the most cost-effective methods to maximize the drainage area treated.

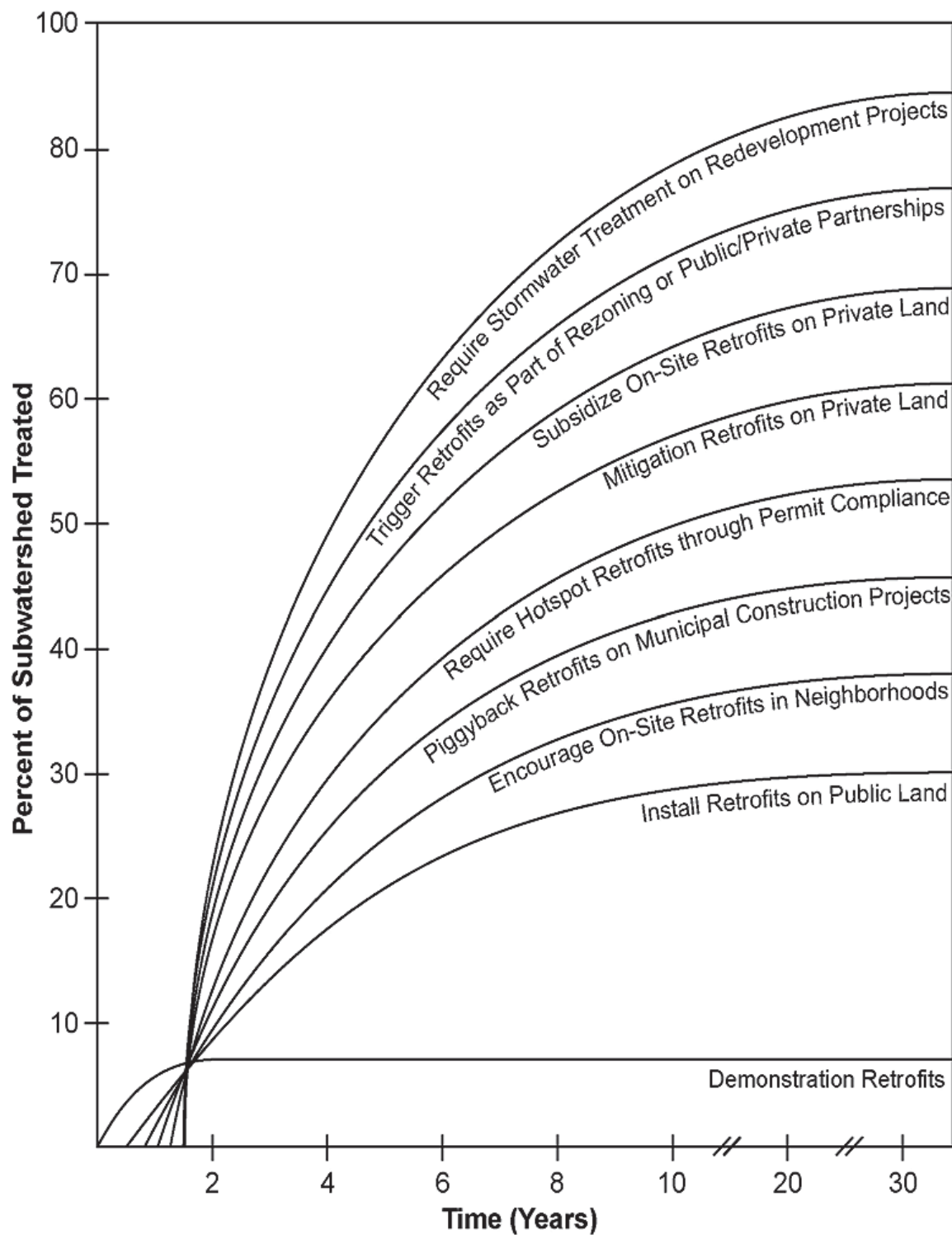


Figure 7.60. Ways to maximize retrofit delivery throughout the watershed

Source: CWP 2007

- ***Transform the Stormwater Maintenance Program.*** Use the local stormwater management maintenance inspection, tracking and enforcement authority to identify potential retrofits and/or significant opportunities to upgrade existing BMPs. This approach can result in identification of opportunities at both public and private stormwater management facilities.
- ***Streamline the Local Government Permitting and Contracting Process.*** Design, engineering, permitting and contracting costs can be 30% - 50% of the total cost of BMP retrofit installations. Project bundling, design/build contracts, call contracts, bid incentives and other project management tools can significantly reduce these costs and improve the quality of the resulting retrofits.

7.11. STRATEGIES TO DELIVER RETROFIT PROJECTS AT THE SUBWATERSHED LEVEL

Subwatershed retrofitting is a major long-term commitment where dozens or even hundreds of individual retrofit projects are built over a multi-year timeframe. As previously noted, retrofitting can be quite costly and is normally the single largest expense involved in watershed restoration. Given the large number of retrofit projects, their high cost and the long timeframe over which they are built, it is important to discuss the strategies on how retrofits can be delivered in a widespread manner.

This section describes a multi-pronged strategy to sustain public investment in retrofitting over many years. The strategy involves multiple ways to deliver retrofits on both public and private land. Many stormwater managers mistakenly believe that retrofitting primarily involves capital construction projects built on public land. Much greater subwatershed coverage, however, can be achieved by a creative combination of financing, education, subsidies, permit coordination and stormwater regulations. To some extent, the retrofit delivery methods are sequential in nature – the first methods are easier to implement early; whereas, latter methods provide expanded treatment in the future.

7.11.1. Demonstration Retrofits

Demonstration Retrofits are the usually the first retrofit delivery method. The best sites are located on public land that is highly visible or receives heavy foot traffic, such as community parks, greenway trails, local schools or the city hall. Severe municipal hotspots, such as public works yards, may also be good candidate sites. Demonstration retrofits are normally financed by state or federal water quality grants. Demonstration retrofits can be installed at any stage of the retrofit process, particularly when they can test a new or innovative retrofit technique.

Although demonstration retrofits serve only a small fraction of subwatershed area, they are an excellent early action project for several reasons. First, retrofits can educate residents about urban stream impacts and restoration potential through interpretive signs, tree planting and other stewardship measures. Second, demonstration retrofits show restoration partners and stakeholders what the retrofit “product” looks like, which helps to increase community acceptance for future projects. Third, demonstration projects enable local agency staff to gain

valuable retrofit design and construction experience that can be used to deliver other retrofits later.

7.11.2. Retrofits on Public Land

The next retrofit delivery method involves construction of storage retrofit projects on public land in the subwatershed. These retrofits are typically located in stream valleys, parks, public right of way and publicly-owned stormwater infrastructure. Public land retrofits are easier to deliver because they do not require land acquisition and can provide community benefits. Storage retrofits are preferred because they can cost-effectively treat the greatest fraction of subwatershed area. Experience has shown that it is possible to treat as much as 30 to 50% of a subwatershed through public land retrofits, particularly if the community owns land in the stream corridor. **Appendix 7-A** provides a case study about the City of Charlottesville's program of retrofitting stormwater management practices on public lands.

Most public retrofits are financed by long-term capital construction budgets dedicated to retrofits or waterway improvements. Consequently, it may take a decade to construct all of the feasible public land retrofits. This phase of retrofit delivery also requires an agency commitment to efficiently manage construction of multiple retrofit projects over time. Another good retrofit strategy is to integrate retrofits into ongoing municipal stormwater maintenance programs, particularly if the facilities are located on public land. The capital budget for stormwater maintenance can be modified to allocate funds to retrofit older ponds to improve their performance at the same time major maintenance problems are being corrected.

7.11.3. Encourage On-site Retrofits in Neighborhoods

This phase of retrofit delivery educates homeowners to persuade them to install low cost on-site retrofits on their property, such as rooftop disconnections, rain barrels or rain gardens. The most effective campaigns educate the public about need to restore watersheds, provide some simple construction tips, and direct interested residents where they can get more specific information and technical assistance. Local governments may wish to hire local watershed groups to "retail" technical assistance directly to neighborhoods and community associations. While it is doubtful that more than 5% of subwatershed residents will install on-site retrofits though education alone (see CWP, 2005a), the relatively low cost of the education program and its outreach and awareness benefits make it a good delivery investment at the outset of the retrofitting process.

7.11.4. Bundle Retrofits into Municipal Construction Projects

The next method incorporates retrofit delivery into other municipal construction capital projects. Communities are constantly investing in streetscaping, transportation projects, school construction, park improvements, water and sewer line rehabilitation, drainage improvements and neighborhood revitalization. The strategy is to bundle retrofits into routine capital projects. In some cases, the match is relatively easy, e.g., including a storage retrofit as part of a culvert upgrade or installing water quality features into drainage improvements. Other bundled retrofits require much greater interagency education and coordination efforts since many agencies do not consider watershed restoration as part of their primary mission. The bundling strategy is

definitely worth the effort since capital budgets for other municipal construction categories exceed water resource spending by a factor of 100 to 500 (U.S. Census, 2006). The largest municipal construction categories include schools, roads, water supply and wastewater treatment, parks and recreation and municipal building.

While some agencies may initially resist efforts to incorporate retrofits into their capital budgets, several recent trends may make it more appealing. First, many units of local government are now subject to municipal stormwater permits and are no longer exempt from treating the quality of the stormwater produced by their construction projects. Bundling retrofits into existing construction projects makes stormwater compliance easier. Second, municipal project managers are often subject to the same environmental permitting requirements as the private sector, and may find that constructing retrofits conveniently meets their off-site mitigation needs. Third, many communities have formally adopted policies to promote sustainable development and/or low impact design practices in their own municipal construction projects. Several progressive communities, such as Santa Monica, CA and Austin TX, have specified a minimum set-aside for construction of on-site stormwater retrofits in their municipal contracting process (CWP, 2006).

7.11.5. Require Hotspot Retrofits Through Permit Compliance

Stormwater hotspots deserve special attention when it comes to retrofit delivery, given their severe water quality impacts and unique regulatory status. The goal is to construct on-site retrofits to treat the quality of runoff from all severe stormwater hotspots in a subwatershed, using existing authority under industrial and/or municipal stormwater permits (see Retrofit Profile Sheet OS-7 in CWP, 2007a). The basic argument is that hotspot runoff violates water quality standards and warrants immediate treatment. Hotspot retrofits are identified based on two systematic levels of subwatershed field inspection – a Hotspot Site Investigation (HIS) to identify severe hotspots (CWP, 2005a) and a more intensive Hotspot Compliance Inspection (HCI) to determine whether a structural retrofit is needed to treat hotspot runoff at the site (CWP, 2005b). In this case, the cost of retrofitting is borne by the hotspot owner, although the locality may also incur costs to find them and enforce compliance.

Stormwater managers should carefully review their existing water quality or illicit discharge ordinances to determine if they actually possess the authority to inspect and enforce compliance over the full range of hotspot sites expected in a subwatershed. If not, local ordinances should be revised to provide for this manner of retrofit delivery. Since many hotspots are small businesses, communities should also consider non-regulatory tools to improve compliance, including employee training, technical assistance and even cost-sharing (CWP, 2005a).

7.11.6. Mitigation Retrofits on Public or Private Land

This method of retrofit delivery matches the mitigation needs of private and quasi-public entities to specific storage retrofits in the subwatershed. As might be imagined, this retrofit delivery method requires exceptional interagency communication and coordination. Developers, highway agencies, utilities and others often seek opportunities to meet offsite environmental mitigation needs (wetlands, water quality trading, stormwater fees or permit conditions). Existing projects

in the subwatershed retrofit inventory can be extremely attractive to permit applicants since the feasibility of the projects is already established and they are located on public land.

Over time, stormwater managers should strive to integrate their retrofit program with any stormwater mitigation, water quality trading or wetland banking efforts that may exist in the community. Most water quality experts predict that water quality trading systems will be common in the future as a cost-effective way to meet TMDLs, wastewater permits or regional nutrient limits. Care should be exercised with mitigation retrofits since they have the potential to be a zero-sum gain, particularly when both the impact and the mitigation occur in the same subwatershed (i.e., the benefit of the mitigation is cancelled out by the impact from the mitigated project). Also, the retrofitting agency may be hesitant about inheriting costly monitoring or maintenance conditions specified in a mitigation permit.

7.11.7. Subsidize On-site Retrofits on Private Land

This retrofit delivery method involves targeted programs to subsidize landowners to install on-site retrofit practices on private land. Such programs go beyond mere education and normally include targeted direct technical assistance and economic incentives to make them happen. The cost of this retrofit delivery method may equal the cost of constructing several large storage retrofits, and may be financed either through grants, operating funds, or a line item in the capital budget.

About a dozen communities have subsidized on-site retrofit delivery at the neighborhood level, primarily to disconnect rooftop runoff from the combined sewer system. Neighborhood adoption rates as high as 15 to 50% have been reported, depending on the extent of the subsidy and the convenience of the retrofit (Profile Sheet OS-10 in Schueler, 2007). Economic incentives include direct cash subsidies, tax credits, discounts on water bills or stormwater utility fees, municipal installation, and provision of free rain barrels.

7.11.8. Trigger Retrofits as Part Public/Private Partnership

Local governments are often a major financial partner in redevelopment and rezoning projects designed to promote neighborhood or commercial revitalization. The community may subsidize development by granting payment in lieu of taxes, tax credits, low interest financing or parcel acquisition. Given the taxpayer investment in these development partnerships, the public should expect that these projects will incorporate sustainable stormwater practices and landscaping features to enhance their community benefit. Consequently, stormwater managers should maximize the use of on-site retrofits during urban design to make sure the final projects are compatible with the water quality goals of the subwatershed plan. These retrofit opportunities seldom appear in the retrofit inventory, so stormwater managers will need to frequently coordinate with local urban planners and economic development agencies to find the best targets of opportunity.

7.11.9. Require Stormwater Treatment for Redevelopment Projects

If a subwatershed still has considerable development potential, stormwater managers should make sure they are imposing the most stringent stormwater criteria possible so that increased pollutant loads generated by new development do not offset loads reduced through retrofitting. If existing stormwater quality criteria are outdated, stormwater managers should update local stormwater criteria to maximize pollutant removal performance.

The infill and redevelopment process provides an excellent opportunity to achieve stormwater treatment where it previously did not exist. The amount of subwatershed treatment that can be achieved by imposing redevelopment stormwater criteria is impressive over the long run. The urban landscape is in constant flux, with sites being continually vacated, demolished and redeveloped all the time. The same is true with public infrastructure. The design or service life of most structures and infrastructure is measured in decades, e.g., buildings (50 to 60 years), parking lots (20 to 30 yrs), bridge decks (40 to 50 yrs) and drainage infrastructure (30 to 50 yrs).

Thus, over several decades, it is quite likely that a sizeable fraction of every subwatershed will undergo redevelopment, infill, or infrastructure rehabilitation. Each of these represents an opportunity to retrofit stormwater treatment into the urban landscape. Therefore, an effective retrofit delivery strategy requires redevelopment and infill projects to address stormwater treatment in some manner. Guidance on developing effective and flexible stormwater treatment criteria for redevelopment projects can be found in CWP (2007b).

Most communities are reluctant to impose more stringent stormwater criteria because of the small size, sharply higher compliance costs, and physical constraints facing redevelopment projects. While on-site compliance is difficult, it does not imply that stormwater treatment criteria should be waived. Rather, it means that special stormwater criteria need to be developed for redevelopment projects that provide incentives to reduce impervious cover, increase forest cover, or promote the use of smart site practices during redevelopment (CWP, 2004a). Local stormwater managers may want to consider a fee-in-lieu approach at redevelopment and infill sites. The basic concept is to waive on-site stormwater requirements in exchange for a fee that is used by the local stormwater authority to build retrofit storage elsewhere in the subwatershed. The fee is usually derived based on the cost to retrofit an equivalent acre of impervious cover using a more economical storage retrofit. In other cases, the fee-in-lieu is based on the average cost to remove a pound of nutrients. Several communities have adopted a fee-in-lieu as an equitable and cost-effective way to treat runoff from small urban sites. Guidance on setting an appropriate fee schedule can be found in Winer (2003).

7.12. REFERENCES

Aird, Janet. March-April, 2009. "Urban Retrofitting." *Stormwater Magazine*. Published by Forester Communications, Santa Barbara, CA.

Atlanta Regional Commission (ARC). 2001. *Georgia Stormwater Management Manual*. Prepared by AMEC, the Center for Watershed Protection, Debo and Associates, Jordan Jones and Goulding, and the Atlanta Regional Commission. Atlanta, Georgia.

Calhoun, A.J.K. and M.W. Klemens. 2002. *Best development practices: Conserving pool-breeding amphibians in residential and commercial developments in the northeastern United States – MCA Technical Paper No. 5*. Metropolitan Conservation Alliance, Wildlife Conservation Society. Bronx, NY.

Cappiella, K. and K. Brown. 2001. *Impervious Cover and Land Use in the Chesapeake Bay Watershed*. Chesapeake Research Consortium. Center for Watershed Protection. Ellicott City, MD

Cappiella, K., T. Schueler, and T. Wright. 2006a. *Urban Watershed Forestry Manual, Part 2: Conserving and Planting Trees at Development Sites*. USDA Forest Service. Newtown Square, PA.

Center for Watershed Protection (CWP). 2000. *The Practice of Watershed Protection*. Ellicott City, MD.

Center for Watershed Protection (CWP). 2004. Redevelopment roundtable consensus document. Ellicott City, MD

Center for Watershed Protection (CWP). 2005a. *Manual 8 of the Urban Subwatershed Restoration Manual Series: Pollution source control practices*. Ellicott City, MD.

Center for Watershed Protection (CWP). 2005b. *Manual 2 of the Urban Subwatershed Restoration Manual Series: Methods to Develop Restoration Plans for Small Urban Watersheds*. Ellicott City, MD.

Center for Watershed Protection (CWP). 2005b. *Manual 11 of the Urban Subwatershed Restoration Manual Series: Unified Subwatershed and Site Reconnaissance – A User's Manual (Ver. 2.0)*. Ellicott City, MD.

Center for Watershed Protection (CWP). August 2007a. *Urban Stormwater Restoration Manual Series – Manual 3: Urban Stormwater Retrofit Practices, Version 1.1*. Ellicott City, MD.

Center for Watershed Protection (CWP). 2007b. *Post Construction Program Building Manual*. Ellicott City, MD.

Center for Watershed Protection (CWP). 2008. *Manual 9 of the Urban Subwatershed Restoration Manual Series: Municipal Pollution Prevention/Good Housekeeping Practices, Version 1.0*. Ellicott City, MD.

Chesapeake Stormwater Network (CSN). 2011. *CSN Technical Bulletin No. 9: Nutrient Accounting Methods to Document Local Stormwater Load Reductions in the Chesapeake Bay Watershed, Ver. 1.0*. Baltimore, MD.

Connecticut Department of Environmental Protection (DEP). 1995. *Connecticut Stormwater Quality Planning: A Guide for Municipal Officials and Regional Planners (draft)*. Bureau of Water Management. Planning and Standards Division. Hartford, CT.

Connecticut DEP. 2004. *Connecticut Stormwater Quality Manual*. Hartford, CT.

Federal Interagency Stream Restoration Working Group. 1998. *Stream Corridor Restoration Principles, Processes, and Practices*.

Law, N. 2006. Research in support of an interim pollutant removal rate for street sweeping and storm drain cleanout. *Technical Memo No. 2*. Prepared for Urban Stormwater Working Group of the EPA Chesapeake Bay Program. Center for Watershed Protection. Ellicott City, MD.

Maryland Department of the Environment (MDE). June 2011. *Accounting for Stormwater Wasteload Allocations and Impervious Acres Treated: Guidance for NPDES Stormwater Permits*. Draft. Baltimore, MD.

Natural Resources Defense Council (NRDC). 1999. *Stormwater Strategies: Community Responses to Runoff Pollution*.

New Jersey Department of Environmental Protection (NJDEP). May, 2000. *Revised Manual for New Jersey: Best Management Practices for Control of Nonpoint Source Pollution from Stormwater, Fifth Draft*.

Pennsylvania Association of Conservation Districts, Keystone Chapter Soil and Water Conservation Society, Pennsylvania Department of Environmental Protection, and Natural Resources Conservation Service. 1998. *Pennsylvania Handbook of Best Management Practices for Developing Areas*. Prepared by CH2MHILL.

Pitt, R. 1999. Department of Civil and Environmental Engineering, University of Alabama at Birmingham.

Rafter, Dan. "Urban Stormwater Retrofit Projects." *Stormwater Magazine*. September, 2008. Published by Forester Communications, Santa Barbara, CA.

Schueler, T. 1987. *Controlling Urban Runoff: A Manual for Planning and Designing Urban Stormwater Best Management Practices*. Metropolitan Washington Council of Governments. Washington, DC.

Schueler, T.R., Kumble, P.A., and M.A. Heraty. 1992. *A Current Assessment of Urban Best Management Practices: Techniques for Reducing Non-Point Source Pollution in the Coastal Zone*. Department of Environmental Programs, Metropolitan Washington Council of Governments. Washington, D.C.

Schueler, T. 2007. *Manual 3 of the Urban Subwatershed Restoration Manual Series: Urban Stormwater Retrofit Practices*. U.S. EPA. Center for Watershed Protection. Ellicott City, MD.

Schueler, T. and Cecilia Lane. 2012. *Technical Memo: Recommendations of the Expert Panel to Define Removal Rates for Urban Stormwater Retrofit Projects*. For the Urban Stormwater Workgroup, U.S. EPA Chesapeake Bay Program. Annapolis, MD.

Winer, R. 2003. *Critical Area 10% Rule Guidance Manual: Maryland Chesapeake and Atlantic Coastal Bays*. Critical Area Commission. Annapolis, MD.

Appendix 7-A

Retrofit Case Study: Charlottesville, Virginia

Stormwater Stewardship on Public Lands

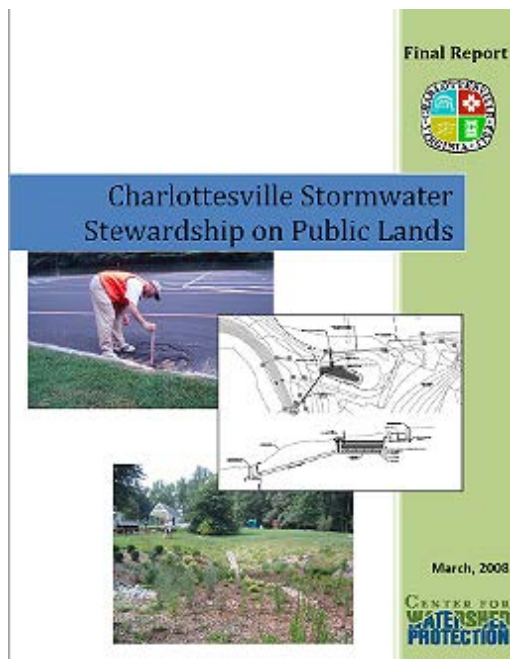


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7-A.1.0. INTRODUCTION Stormwater Stewardship on Public Lands Study

The City of Charlottesville is located in the Central Virginia Piedmont at the foothills of the Blue Ridge Mountains (**Figure 7-A.1**). Charlottesville is the urban center of the Rivanna River watershed (**Figure 7-A.2**), which flows into the James River, a major tributary to the Chesapeake Bay. **Figure 7-A.3** shows the Rivanna River, which flows along the eastern edge of Charlottesville, and is the focal point of local stormwater and watershed management activities.



Figure 7-A.1. Location of Charlottesville



Figure 7-A.2. Location of Rivanna River Watershed



Figure 7-A.3. Rivanna River at Charlottesville

At only 10.4 square miles and 40,000 residents, Charlottesville is a fairly dense city. As Charlottesville is home to the University of Virginia, the city becomes even more dense when school is in session and an additional 20,000 students are in residence.

The City is mostly built out. The majority of development took place without the benefit of stormwater management requirements and in the absence of the current understanding of stormwater's relationship to water quality and watershed health. This has led to major impacts to the City's urban streams, including severe streambank erosion (**Figure 7-A.4**) and associated sedimentation. Several local streams do not meet water quality standards and have TMDLs established. We also recognize that we are part of the Chesapeake Bay watershed, and that we not only have a responsibility for helping to restore the health of our own local waterways, but that we are also part of larger, significant, and very important restoration efforts to save the Bay.



Figure 7-A.4. Streambank Erosion in Charlottesville

Charlottesville is also a City that has made several formal commitments to environmental stewardship, including adoption of an Environmental Sustainability Policy and Green City Vision statement, and becoming a signatory to the U.S. Mayors Climate Protection Agreement. City policies also pursue protection of riparian buffers, energy and water conservation and efficiency, stream restoration, urban forestry, recycling, greening of the City vehicle fleet, and public transportation initiatives.

On top of all that, Charlottesville is also a MS4 Phase II community. As such, the City has held a stormwater discharge permit from the Virginia Department of Environmental Quality (DEQ) since 2003 and has developed and implemented a stormwater management program that addresses the six minimum control measures required by the permit. City officials have found that stormwater retrofits can be an effective strategy for addressing various components of the overall stormwater management program.

Beyond historical impacts and environmental commitments, current stormwater management regulations in place address impacts from new development in the City. The regulations provide incentives and requirements for the use of low impact development (LID) practices, and other water quality BMPs are being implemented for the remaining development area. Incorporating innovative BMPs into new City owned construction projects is also a priority. However such opportunities are limited, since the City is not undertaking large amounts of new construction.

As a result of all these factors, City officials recognized years ago the importance of installing innovative BMPs as retrofits on existing public lands. They viewed these retrofits as an effective means of addressing historic impacts, providing for environmental stewardship, and meeting MS4 permit commitments and water quality goals. Implementation of retrofit projects on public lands also provides an opportunity to demonstrate and promote the use of the best new practices. The hope is that these practices will become widely used in new development and redevelopment projects and at other existing development sites. Now, as of 2013, with retrofits of existing developed lands becoming a requirement of the City's MS4 permit, they realize that past experience with and proactive engagement on the retrofit front has positioned them well to meet the regulatory requirements related to the restoration of the Chesapeake Bay.

7-A.2.0. GREENLEAF PARK RAIN GARDEN PILOT RETROFIT PROJECT

The retrofit ball got rolling in Charlottesville in 2005. A potential grant opportunity served as the impetus for a rudimentary first look at a subset of public lands with the objective of identifying potential retrofit opportunities. After evaluating seven properties (without the benefit of a distinct methodology, established performance goals or scientific evaluation criteria), the City's Greenleaf Park was selected as the best candidate for a retrofit. City officials decided to install a rain garden.

The City applied for and received a grant from EPA's Chesapeake Bay Program and the DEQ. After contracting out the rain garden design, City Park crews created the landscape plan and constructed the 1,200 square foot rain garden. This project provided the first retrofitting tie-in to the City's MS4 permit, addressing minimum control measures 1 and 2 (public education, outreach, and involvement). Nearly fifty volunteers assisted in this project, accomplishing tasks such as grading, laying filter fabric, mulching, and planting rain garden vegetation. A webpage tracked the

construction process, and permanent, high quality educational signage was installed (see **Figure 7-A.5** below).



Figure 7-A.5. Greenleaf Park Rain Garden Retrofit Project

Since its completion, this retrofit has also served as the focus of numerous interpretive tours for the public, an outdoor classroom for a local elementary school, the site of environmental training for City staff, and the backdrop for interviews and environmental news stories by the local media. Hundreds of community members have directly interacted with this retrofit project.

The success of the project inspired City officials to consider additional retrofit opportunities. However, they also realized the need to undertake a methodical, strategic, and scientifically defensible examination of public lands in order to identify the most effective, appropriate, and feasible retrofits that would improve water quality and engage the community in local watershed stewardship.

7-A.3.0. “STORMWATER STEWARDSHIP ON PUBLIC LANDS” STUDY

City officials determined that a formal study was needed, and they gave it the title of *Stormwater Stewardship on Public Lands*. They limited the scope of the study to City Parks and school campuses, because they wanted the retrofits to be publicly accessible in order to best serve as demonstration projects and interactive learning opportunities. Early in the process, they engaged the following key partners:

- Charlottesville City Schools
- The following agencies of City government:
 - Public Works
 - Environmental Sustainability
 - Parks and Recreation
 - Neighborhood Development Services

The partners collaborated on a grant proposal, which was awarded by the National Fish and Wildlife Foundation. The Center for Watershed Protection was selected as the contractor to perform the Study, which was a perfect fit given their recent completion of a manual entitled *Urban Stormwater Retrofit Practices*.

7-A.3.1. Scope and Overall Objectives of the Study

The first step was to develop the overall objectives of the study. The major study objectives included a systematic evaluation of the City's parks and school campuses, in order to identify a prioritized set of retrofits which would improve water quality by removing pollutants, increasing groundwater recharge, and decreasing stormwater volume and velocity. The end result would be a catalogue of retrofit opportunities tailored to the site conditions in and around the public lands. These projects could also serve as stormwater education and outreach opportunities. This study was envisioned as the blueprint and guide for future City retrofit efforts.

7-A.3.2. Development of Retrofit Performance Goals

Early in the process, performance goals for the retrofits that they would be searching for were also established. Care was taken to ensure that the specific retrofit performance goals were complimentary to and would help achieve the overall study objectives.

Primary goals included, but were not limited to:

- **Pollutant removal**, with the focus on local and regional pollutants of concern. Sedimentation had been identified as one the greatest threats to the health of the Rivanna River watershed, and several local streams were impaired due to contamination by bacteria. Also, nitrogen and phosphorus are the nutrients targeted for reductions in Chesapeake Bay restoration efforts.
- **Runoff reduction**: the retrofits should achieve overall reductions in stormwater volume.
- **Relief of drainage problems**, where possible.
- **Education and Outreach**: the projects needed to be in accessible locations where school children and staff, the general public, and the development community can interact with and learn from the retrofits.

Secondary Goals included, but were not limited to:

- **Naturalization and recreation**, ensuring that there was a sensitivity to existing or future active recreational programs on the public lands.
- **Quick Implementation**: it was important to identify several retrofits that could be implemented right away in order to capitalize on the momentum created by the study.
- **Create wildlife habitat** and **protect/enhance tree canopy coverage** in the City.

These performance goals were used during the field work portion of the study to evaluate and prioritize potential retrofit projects. This led to identification of 24 properties with high and medium retrofit potential, and some sites with low retrofit potential (**Figure 7-A.6**). The top 24 properties were prioritized for fieldwork. This prioritization was essential in order to focus the subsequent field work on the best opportunities to make meaningful improvements on the public lands themselves and in the subwatersheds in which they are situated.

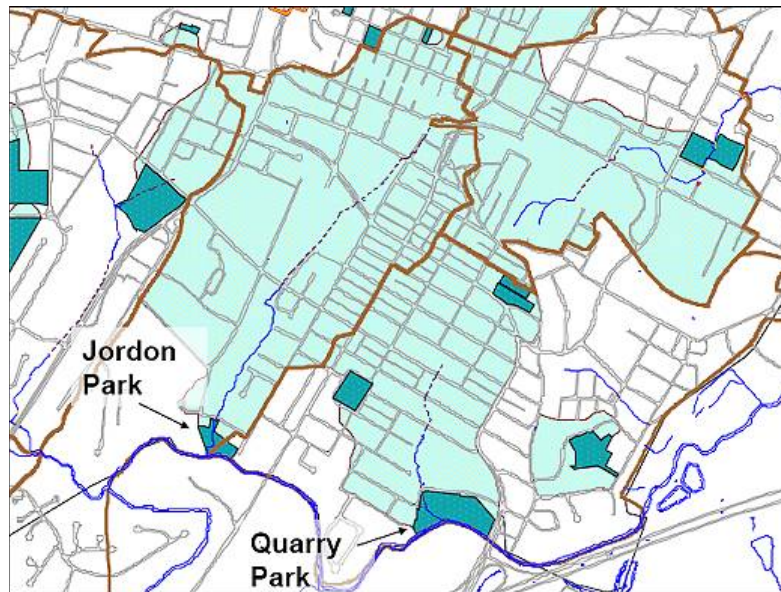


Figure 7-A.6. Map of Portion of Study Area with Potential Project Locations

7-A.3.3. Field Investigations

The 24 sites were evaluated over the course of three days (**Figures 7-A.7 and 7-A.8**). Field teams were led by a CWP staff person experienced with retrofitting. The teams included other Center and City staff as well.



Figure 7-A.7. Retrofit Reconnaissance



Figure 7-A.8. Finding Retrofit Opportunities

City staff participated to learn the process in order to be able to replicate it in the future, glean as much insight into the potential retrofits as possible, and to offer historic and current knowledge of the properties (such as ongoing master plans or programming conflicts). Detailed field maps, the Center's Retrofit Reconnaissance Investigation (RRI) form, and the methodology from the CWP's Retrofitting Manual were used throughout the field investigations. The City staff was also interested in connecting the field investigations to the MS4 program's minimum control measure 3 (illicit discharge detection and elimination). Staff watched for illicit discharges from the public properties or from surrounding private properties. One active discharge was found and addressed.

A total of 121 potential projects were identified, including 74 retrofits, many of which had concept sketches drawn in the field. These projects involved the following list of:

- 74 retrofit projects (8 storage and 66 on-site practices) that included:
 - Bioretention /rain gardens
 - Stormwater planters
 - Impervious cover removal
 - Swales
 - Vegetated roof
 - Rainwater harvesting
 - Forested stormwater wetland
- 26 landscape management / stewardship projects (e.g., tree planting, use of native landscaping, designation of "no mow" areas, repair of bare soil and eroded areas, etc.)
- 8 pollution prevention projects (e.g., improved dumpster management and materials storage, street sweeping, and clean up of trash and debris)
- 13 outfall repair/stabilization projects
- Identification of important unmapped features (e.g., stormwater infrastructure and live streams, etc.)

7-A.3.4. Ranking Potential Projects

Once the field work was completed and all the potential projects had been identified, it was time to evaluate and rank them. A series of primary and secondary screening factors were derived directly from the initial performance goals and refined through several meetings with City and CWP staff. Each factor was assigned a weight to produce a 100-point scale for ranking, with the primary factors carrying more weight. The ranking process only applied to the stormwater retrofit projects, because these could be directly compared to each other based on pollutant load reductions, cost-effectiveness, and the other screening factors. Other projects (e.g., pollution prevention and landscape stewardship) also reduce pollutant loads, but they are more difficult to quantify and in many cases can be built into existing operation and maintenance activities. Therefore they were not ranked, but the City staff still plan to pursue them over time.

The primary screening goals used in ranking projects were as follows:

- Amount of Total Phosphorus (TP) removed (used as an indicator for other pollutants as well)
- Cost effectiveness (cost per pound of pollution removed)
- Amount of capture of off-site runoff
- Amount of runoff volume reduction
- Relative opportunity for outdoor learning / community outreach

The secondary screening goals included, but were not limited to the following:

- Whether bacteria removal was accomplished by the project
- Degree to which the project promotes LID / innovative practices
- Degree to which the project has a lower maintenance burden and cost

After all the retrofits had been ranked against the screening goals, a meeting was held with CWP and City staff to review the ranked projects and discuss the strongest candidates for design work. Several factors were discussed, including the following:

- Project rank according to the screening factors
- Representation of different types of BMPs
- Representation across parks and school sites
- Sites that provided opportunities for early implementation or already had master plans for development
- Ease of implementation vs. impediments to implementation (e.g., permitting, public safety concerns, utility conflicts, etc.).

Local officials admit that common sense and informed intuition also played a role in prioritizing projects for action. Subsequently, the top candidate sites were evaluated in the field and, through a series of discussions, the final set of candidate sites and retrofit projects were selected.

The evaluation and ranking process put the potential projects in context and produced a prioritized list (see **Figure 7-A.9**).

| Site Name | Description | Project ID | TOTAL |
|-----------------------------|-------------------------------------|------------|-------|
| Meadowcreek Park | cistern from hotel parking lot | MW-01 | 93 |
| Azalea Park | stormwater wetland | AZ-02 | 89 |
| Schenks Greenway | rain gardens along path | SC-03 | 84 |
| Buford School | bioswale @ pool building | BU-02 | 82 |
| Charlottesville High School | rain garden @ MLK PA center | CHS-06 | 82 |
| Johnson School | bioswale @ amphitheatre | JO-01 | 77.5 |
| Jordan Park | expanded buffer, divert road runoff | JR-01 | 75 |
| Pen Park | bioretention at parking lot | PP-03 | 74 |
| Forest Hills Park | daylight pipe, create swale | FH-01 | 73 |
| Tonsler Park | forested wetland | TO-01 | 73 |
| McIntire Park – West | bioretention, small parking lot | MPW-02 | 69 |
| Pen Park | bioretention at parking lot | PP-06 | 69 |
| McIntire Park – West | bioretention | MPW-01b | 68 |
| Tonsler Park | rain gardens | TO-02a | 67 |
| Venable School | disconnect downspouts | VN-02c | 67 |
| Venable School | disconnect downspouts | VN-02b | 65.5 |
| Johnson School | IC removal | JO-02b | 64.5 |
| Burnley-Moran School | swale @ asphalt channel | BM-02b | 64 |
| Jackson-Via School | bioretention, swale | JV-02a/b | 63 |
| Burnley-Moran School | swale for roadway | BM-01 | 62 |
| Clark School | rain gardens @ playground | CS-02b | 62 |
| Rives Park | rain gardens | RI-01 | 62 |

Figure 7-A.9. Ranked List of Potential Retrofit Projects

The prioritized list will serve as a guide for future retrofit implementation, and City staff will be able to quickly ascertain which projects best meet the established performance goals. However, the list is only a guide. Professional judgment, funding opportunities, and broader City needs also will play a role in determining which projects will be implemented at any given time.

7-A.3.5. Formal Project Designs

As one of the final steps in the Study, formal designs were developed for eight selected retrofit projects. In some cases, the City's Parks Department will continue to refine and ultimately implement the landscape plan details, which began only as concept plans.

The design work included the following:

- Preliminary plan view and profile plans
- Landscape concept plans
- Preliminary material specifications and quantities
- Cost estimates
- Concept grading plans

The eight retrofit projects that were designed included the following:

- Two rainwater harvesting projects at City schools
 - One at Venable Elementary School, where roof water will be collected and used by students and staff to irrigate on-site landscaping features.
 - The other project, which was installed in 2009, was a 40,000 gallon underground rainwater harvesting cistern at Charlottesville High School (**Figures 7-A.10 and 7-A.11**) that captures stormwater from approximately 26,000 square feet of the nearby Performing Arts Center roof. The cistern provides water to a pre-existing irrigation system for one of the high school's ball fields.



Figure 7-A.10. High School Athletic Field before Rainwater Harvesting Retrofit



Figure 7-A.11. Construction of the Underground Rainwater Harvesting Cistern at the High School

- Two bioretention / rain garden features
 - One at Burnley-Moran Elementary School, which will convert an existing asphalt drainage channel (**Figure 7-A.12**) that conveys one acre of parking lot and roof runoff into a series of cascading step-pool rain gardens connected by rock spillways. It will be adjacent to a new outdoor classroom, allowing for great educational opportunities.



Figure 7-A.12. Existing Asphalt Swale to be Converted

- The other was constructed at Charlottesville High School in 2009. It is a 2,600 square foot bioretention filter that treats runoff from four acres of parking lot and adjacent residential parcels at the entrance of the co-located Performing Arts Center (**Figure 7-A.13**). The bioretention filter is used by the high school science classes as an outdoor classroom, and stormwater quality and flow monitoring has been conducted by the University of Virginia and the Rivanna River Basin Commission.



Figure 7-A.13. Charlottesville High School Bioretention Filter

- One bio-swale (Dry Swale), which was constructed in 2009 in Forest Hills Park that daylighted about 300 feet of the existing stormwater piping just prior to where it outfalls into an intermittent stream. The swale treats on-site as well as off-site drainage from nearly 13 acres.
- Constructed Stormwater Wetlands in Azalea Park (**Figures 7-A.14 and 7-A.15**), which were constructed in 2013. The wetlands treat runoff from 40 acres of a residential neighborhood with no pre-existing stormwater treatment, just prior to it entering one of the City's major waterways for which a TMDL has been established.



Figure 7-A.14. Azalea Park Before Constructed Wetland



Figure 7-A.15. Azalea Park After Constructed Wetland

- A piedmont outfall repair – a typical design of step pool treatment cells that provide stable conveyance, stormwater treatment, and volume reduction. The typical design is meant to be customized and applied to various sites where there is an outfall in need of stabilization and repair.
- A runoff diversion – another typical design meant to be customized and applied to various sites where there is an opportunity to divert runoff from an untreated area, such as a roadway, to a riparian buffer or bioretention area.

In addition to the projects designed as part of the study, the City has pursued several other retrofit opportunities. When the Parks Department's maintenance yard was revamped, they took the opportunity to install a bioretention filter to provide stormwater treatment where there had been none previously. A block of City sidewalk has been replaced with permeable pavers and an ADA parking stall was replaced with pervious concrete. Furthermore, the City converted 9,000 square feet of roof area on the City Hall building to a vegetated roof when the building's maintenance cycle called for replacing the roof (see **Figures 7-A.16 and 7-A.17** below).



Figure 7-A.16. Original Roof on City Hall



Figure 7-A.17. Replacement Vegetated Roof Area on City Hall

The City's Public Works Yard has had two retrofits installed. A small 1,500 gallon cistern was installed to collect rainwater off of the fleet maintenance garage, and the water is used in City street sweepers and flusher trucks (see **Figures 7-A.18 and 7-A.19**). Catch basin filter inserts designed to remove debris, sediment, and hydrocarbons have also been installed in strategic locations throughout the property (**Figure 7-A.20**).



Figure 7-A.18. City Yard Before Retrofit



Figure 7-A.19. City Yard Cistern Retrofit



Figure 7-A.20. City Yard Catch Basin Filter Inserts

Rainwater harvesting systems have also been installed at three Charlottesville City Schools properties to provide irrigation water for schoolyard gardens (**Figure 7-A.21**)



Figure 7-A.21. Rainwater Harvesting at Schoolyard Garden

7-A.4.0. COMMUNITY INVOLVEMENT

The retrofit process and the retrofits in this case study can be seen as an important step in the evolution of a “retrofit culture” (**Figure 7-A.22**) for the City. It is hoped that several high-profile projects on public land will create the spark that leads to ongoing retrofit activities on both public and private land.



Figure 7-A.22. Community Involvement in a Retrofit Culture

The City intends to install many more of the retrofits and practices that have been identified in the Study. This effort ties into a broader Water Resources Protection Program (WRRP), one component of which will provide funding for future retrofit projects. The WRRP will be funded in part through a stormwater utility fee, and includes incentives built into the program for private property owners to construct retrofits on their properties.

As master planning and renovation efforts for park and school properties are undertaken, the Study provides the basis for retrofit opportunities. The Study also affords City officials the potential for private sector involvement (new development and redevelopment projects) through a fee-in-lieu program. Through such a program, new projects that are unable to achieve stormwater management requirements completely on the development site could contribute to a City administered fund that would pay to construct retrofits identified in the Study. In these ways, City officials hope that the retrofit culture can take hold firmly and, over time, result in community benefits as well as real and meaningful improvements in local water quality.

7-A.5.0. CONCLUSION

The following recommendations grew out of the retrofit study experience:

- In order to realize the best results from stormwater retrofits, consider them strategically and methodically, and tailor them to meet the subwatershed's particular needs. Having a reference like the Charlottesville study will enable a community to incorporate retrofits into future planning efforts and broader water resource protection initiatives in the future.
- Retrofits on public lands should be planned for use as demonstration projects to encourage and inspire private property owners to install them on their own property. Doing this increases the local knowledge base regarding innovative stormwater management. The more familiar, understood, and tangible these kinds of projects are, the more likely they are to be replicated elsewhere.
- Involve and educate the public about local retrofit projects. The more involved and invested in a project the community becomes, the greater the chance for achieving long-term success. Using public land, particularly school campuses, allows the greatest access to the retrofits for public interaction and educational opportunities.
- In developed subwatersheds with historical impacts to water resources, such as the watersheds of Charlottesville, installing retrofits is an essential and major component of a successful water resource restoration initiative. In Charlottesville, stormwater retrofits have also become an integral part of the City's overall environmental sustainability efforts.

The Charlottesville *Stormwater Stewardship on Public Lands* Study can be reviewed in more detail at the following web site:

www.charlottesville.org/environmental

7-A.6.0. REFERENCES

Center for Watershed Protection. August 2007. *Urban Stormwater Restoration Manual Series – Manual 3: Urban Stormwater Retrofit Practices, Version 1.1*. Ellicott City, MD.

Frisbee, Dan. March, 2010. "Stormwater Retrofitting as a Program Tool." Presentation at the conference entitled *From the Rooftop to the Bay: Implementing Stormwater Management Strategies in the Chesapeake Bay Watershed*. Staunton, Virginia. Chesapeake Bay Stormwater Training Partnership.

Appendix 7-B

CWP Retrofit Reconnaissance Investigation Checklist

This Appendix provides a *Retrofit Reconnaissance Investigation (RRI)* checklist form, developed by the Center for Watershed Protection (CWP), that can be used to conduct retrofit reconnaissance in your community. This form was developed in conjunction with the CWP's Manual entitled *Urban Stormwater Retrofit Practices* (2007).

Retrofit Reconnaissance Investigation

RRI

| | | | | | |
|--|--|----------------------|--|------------------------|--|
| WATERSHED: | | SUBWATERSHED: | | UNIQUE SITE ID: | |
| DATE: | | ASSESSED BY: | | CAMERA ID: | |
| GPS ID: | | LMK ID: | | LONG: | |
| SITE DESCRIPTION | | | | | |
| Name: _____ | | | | | |
| Address: _____ | | | | | |
| Ownership: <input type="checkbox"/> Public <input type="checkbox"/> Private <input type="checkbox"/> Unknown If Public, Government Jurisdiction: <input type="checkbox"/> Local <input type="checkbox"/> State <input type="checkbox"/> DOT <input type="checkbox"/> Other: _____ | | | | | |
| Corresponding USSR/USA Field Sheet? <input type="checkbox"/> Yes <input type="checkbox"/> No If yes, Unique Site ID: _____ | | | | | |
| Proposed Retrofit Location: | | | | | |
| Storage <input type="checkbox"/> Existing Pond <input type="checkbox"/> Above Roadway Culvert <input type="checkbox"/> Below Outfall <input type="checkbox"/> In Conveyance System <input type="checkbox"/> In Road ROW <input type="checkbox"/> Near Large Parking Lot <input type="checkbox"/> Other: _____ | | | On-Site <input type="checkbox"/> Hotspot Operation <input type="checkbox"/> Individual Rooftop <input type="checkbox"/> Small Parking Lot <input type="checkbox"/> Small Impervious Area <input type="checkbox"/> Individual Street <input type="checkbox"/> Landscape / Hardscape <input type="checkbox"/> Underground <input type="checkbox"/> Other: _____ | | |
| DRAINAGE AREA TO PROPOSED RETROFIT | | | | | |
| Drainage Area ≈ _____ Imperviousness ≈ _____ % Impervious Area ≈ _____ | | | Drainage Area Land Use: <input type="checkbox"/> Residential <input type="checkbox"/> Institutional <input type="checkbox"/> SFH (< 1 ac lots) <input type="checkbox"/> Industrial <input type="checkbox"/> SFH (> 1 ac lots) <input type="checkbox"/> Transport-Related <input type="checkbox"/> Townhouses <input type="checkbox"/> Park <input type="checkbox"/> Multi-Family <input type="checkbox"/> Undeveloped <input type="checkbox"/> Commercial <input type="checkbox"/> Other: _____ | | |
| Notes: | | | | | |
| EXISTING STORMWATER MANAGEMENT | | | | | |
| Existing Stormwater Practice: <input type="checkbox"/> Yes <input type="checkbox"/> No <input type="checkbox"/> Possible If Yes, Describe: | | | | | |
| | | | | | |
| Describe Existing Site Conditions, Including Existing Site Drainage and Conveyance: | | | | | |
| | | | | | |
| Existing Head Available and Points Where Measured: | | | | | |
| | | | | | |

Retrofit Reconnaissance Investigation

RRI**PROPOSED RETROFIT****Purpose of Retrofit:**

- ☐ Water Quality ☐ Recharge ☐ Channel Protection ☐ Flood Control
☐ Demonstration / Education ☐ Repair ☐ Other: _____

Retrofit Volume Computations - Target Storage:**Retrofit Volume Computations - Available Storage:****Proposed Treatment Option:**

- ☐ Extended Detention ☐ Wet Pond ☐ Created Wetland ☐ Bioretention
☐ Filtering Practice ☐ Infiltration ☐ Swale ☐ Other: _____

Describe Elements of Proposed Retrofit, Including Surface Area, Maximum Depth of Treatment, and Conveyance:**SITE CONSTRAINTS****Adjacent Land Use:**

- ☐ Residential ☐ Commercial ☐ Institutional
☐ Industrial ☐ Transport-Related ☐ Park
☐ Undeveloped ☐ Other: _____

Possible Conflicts Due to Adjacent Land Use? ☐ Yes ☐ No

If Yes, Describe:

Access:

- ☐ No Constraints
 Constrained due to

- ☐ Slope ☐ Space
☐ Utilities ☐ Tree Impacts
☐ Structures ☐ Property Ownership
☐ Other: _____

Conflicts with Existing Utilities:

- ☐ None
☐ Unknown

Yes**Possible**

- | | | |
|--------------------------|--------------------------|--------------------------|
| <input type="checkbox"/> | <input type="checkbox"/> | Sewer |
| <input type="checkbox"/> | <input type="checkbox"/> | Water |
| <input type="checkbox"/> | <input type="checkbox"/> | Gas |
| <input type="checkbox"/> | <input type="checkbox"/> | Cable |
| <input type="checkbox"/> | <input type="checkbox"/> | Electric |
| <input type="checkbox"/> | <input type="checkbox"/> | Electric to Streetlights |
| <input type="checkbox"/> | <input type="checkbox"/> | Overhead Wires |
| <input type="checkbox"/> | <input type="checkbox"/> | Other: _____ |

Potential Permitting Factors:

- | | | |
|------------------------------|-----------------------------------|---------------------------------------|
| Dam Safety Permits Necessary | <input type="checkbox"/> Probable | <input type="checkbox"/> Not Probable |
| Impacts to Wetlands | <input type="checkbox"/> Probable | <input type="checkbox"/> Not Probable |
| Impacts to a Stream | <input type="checkbox"/> Probable | <input type="checkbox"/> Not Probable |
| Floodplain Fill | <input type="checkbox"/> Probable | <input type="checkbox"/> Not Probable |
| Impacts to Forests | <input type="checkbox"/> Probable | <input type="checkbox"/> Not Probable |
| Impacts to Specimen Trees | <input type="checkbox"/> Probable | <input type="checkbox"/> Not Probable |

How many? _____

Approx. DBH _____

Other factors: _____**Soils:**

- Soil auger test holes: ☐ Yes ☐ No
 Evidence of poor infiltration (clays, fines): ☐ Yes ☐ No
 Evidence of shallow bedrock: ☐ Yes ☐ No
 Evidence of high water table (gleying, saturation): ☐ Yes ☐ No

Retrofit Reconnaissance Investigation

RRI

SKETCH

Retrofit Reconnaissance Investigation

RRI**DESIGN OR DELIVERY NOTES****FOLLOW-UP NEEDED TO COMPLETE FIELD CONCEPT**

- | | |
|---|--|
| <input type="checkbox"/> Confirm property ownership | <input type="checkbox"/> Obtain existing stormwater practice as-builts |
| <input type="checkbox"/> Confirm drainage area | <input type="checkbox"/> Obtain site as-builts |
| <input type="checkbox"/> Confirm drainage area impervious cover | <input type="checkbox"/> Obtain detailed topography |
| <input type="checkbox"/> Confirm volume computations | <input type="checkbox"/> Obtain utility mapping |
| <input type="checkbox"/> Complete concept sketch | <input type="checkbox"/> Confirm storm drain invert elevations |
| <input type="checkbox"/> Other: _____ | <input type="checkbox"/> Confirm soil types |

INITIAL FEASIBILITY AND CONSTRUCTION CONSIDERATIONS**SITE CANDIDATE FOR FURTHER INVESTIGATION:**☐ YES ☐ NO ☐ MAYBE**IS SITE CANDIDATE FOR EARLY ACTION PROJECT(S):**☐ YES ☐ NO ☐ MAYBE**IF NO, SITE CANDIDATE FOR OTHER RESTORATION PROJECT(S):**☐ YES ☐ NO ☐ MAYBE

If YES, TYPE(S): _____

Appendix C

Derivation of the Retrofit Pollution Removal Adjustor Curves

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7-C.1.0 INTRODUCTION

In early 2012, the Urban Stormwater Work Group of the USEPA Chesapeake Bay Program formed an expert panel to define pollutant removal rates for urban stormwater retrofit projects. The Panel started by noting the strong relationship between the runoff volume treated and the degree to which runoff reduction is achieved at individual BMPs. The primary source was a comprehensive analysis of runoff reduction and pollutant event mean concentration reduction data for a wide range of BMPs that are typically applied in retrofitting (CWP and CSN, 2008).

7-C.2.0 PROCESS

CSN (2011) developed a general table to determine nutrient removal rates for all classes of retrofits, and this approach was used as a starting point. The basic technical approach defines an “anchor” rate for composite stormwater treatment (ST) and runoff reduction (RR) practices for one inch of runoff treatment (see **Table 7-C.1** below). The RR practices included six different LID practices including bioretention, dry swales, infiltration, permeable pavement and green roofs/rain tanks.

The composite for ST practices included wet ponds, constructed wetlands, sand filters, and wet swales. Dry ponds and Dry ED pond were omitted from the ST category since they have such low removal rates that they are typically not targets of retrofitting. The annual mass nutrient removal rates associated with each practice presented in **Table 7-C.2** below was averaged for the composite practices, as shown in **Table 7-C.1**.

Table 7-C.1. Composite Approach to Derive Nutrient Mass Load Reductions for RR and ST Practices^{1, 2}

| PRACTICE | TP Mass Reduction (%) | TN Mass Reduction (%) |
|---|-----------------------|-----------------------|
| Bioretention | 73 | 77 |
| Dry Swale | 66 | 63 |
| Infiltration | 75 | 78 |
| Permeable Pavers | 70 | 70 |
| Green Roof/Rain Tank | 55 | 55 |
| Average RR | 70 | 70² |
| Wet Ponds | 63 | 35 |
| Constructed Wetlands | 63 | 40 |
| Filtering Practice | 63 | 38 |
| Wet Swale | 30 | 30 |
| Average ST | 55 | 35 |
| ¹ Source: Table A-4, nutrient rates computed using the average mass reduction for both Design Level 1 and Level 2. | | |
| ² This value was subsequently discounted by 18% to reflect the impact of nitrate migration from runoff reduction practices described later in this appendix. | | |

The next step involved using a rainfall frequency spreadsheet analysis from Washington, D.C. to estimate how the anchor removal rate would change based on different levels of runoff capture by the composite practice. The percent of the annual rainfall that would be captured by a retrofit designed for a specific control depth was estimated by summing the precipitation for all of the

Table 7-C.2. Mass Nutrient Removal Rates for Stormwater Practices

| Practice | Design Level ¹ | TN Load Removal ⁴ | TP Load Removal ⁴ |
|---|---------------------------|------------------------------|------------------------------|
| Rooftop Disconnect ⁵ | 1 | 25 to 50 | 25 to 50 |
| | 2 ⁶ | 50 | 50 |
| Filter Strips ⁵ | 1 | 25 to 50 | 25 to 50 |
| | 2 ⁶ | 50 to 75 | 50 to 75 |
| Green Roof | 1 | 45 | 45 |
| | 2 | 60 | 60 |
| Rain Tanks & Cisterns ⁷ | 1 | 15 to 60 | 15 to 60 |
| | 2 | 45 to 90 | 45 to 90 |
| Permeable Pavers | 1 | 59 | 59 |
| | 2 | 81 | 81 |
| Infiltration Practices | 1 | 57 | 63 |
| | 2 | 92 | 93 |
| Bioretention Practices | 1 | 64 | 55 |
| | 2 | 90 | 90 |
| Dry Swales | 1 | 55 | 52 |
| | 2 | 74 | 76 |
| Wet Swales | 1 | 25 | 20 |
| | 2 | 35 | 40 |
| Filtering Practices | 1 | 30 | 60 |
| | 2 | 45 | 65 |
| Constructed Wetlands | 1 | 25 | 50 |
| | 2 | 55 | 75 |
| Wet Ponds ⁸ | 1 | 30 (20) | 50 (45) |
| | 2 | 40 (30) | 75 (65) |
| ED Ponds | 1 | 10 | 15 |
| | 2 | 24 | 31 |
| Notes ¹ See specific level 1 and 2 design requirements within each practice specification ² Annual runoff reduction rate (%) as defined in CWP and CSN (2008) ³ Change in nutrient event mean concentration in and out of practice, as defined in CWP and CSN (2008) ⁴ Load removed is the product of annual runoff reduction rate and change in nutrient EMC ⁵ Lower rate is for HSG soils C and D, Higher rate is for HSG soils A and B ⁶ Level 2 design involves soil compost amendments, may be higher if combined with secondary runoff reduction practices ⁷ Range in RR depends on whether harvested rainwater is used for indoor, outdoor or discharged to secondary runoff reduction practice. Actual results will be based on spreadsheet ⁸ lower nutrient removal parentheses apply to ponds in coastal plain terrain | | | |

storms less than the control depth, plus the product of the number of storm events greater than the control depth multiplied by the control depth. This sum was then divided by the sum of the total precipitation. A visual representation of this may be helpful and can be seen as follows:

$$\% \text{ Annual Rainfall} = \frac{(SUM P_{<CD} + CD(in) * (\# \text{ of Storms } P_{>CD}))}{Sum \text{ of Total Precipitation (inches)}}$$

Where:

$P_{<CD}$ = Precipitation of Storms less than Control Depth (inches)

$P_{>CD}$ = Precipitation of Storms greater than Control Depth (inches)

CD = Control Depth (inches): the depth of rainfall controlled by the practice

Once the percent annual rainfall has been determined for a specific control depth, we can use this along with the anchor pollutant removal rates to determine the pollutant removal values associated with a specific control depth. For example:

$$Pollutant \text{ Removal }_{CD} = \frac{(Pollutant \text{ Removal Value}_{AR} * \% \text{ Annual Rainfall}_{CD})}{\% \text{ Annual Rainfall}_{AR}}$$

Where:

Pollutant Removal Value_{AR} = The anchor rates for N, P or TSS and ST or RR practices per 1.0" of Control Depth (~88% Annual Rainfall)

| Phosphorus | | Nitrogen | | Sediment | |
|------------|-----|----------|-----|----------|-----|
| ST | RR | ST | RR | ST | RR |
| 55% | 70% | 35% | 60% | 70% | 75% |

$\% \text{ Annual Rainfall }_{CD}$ = The $\% \text{ Annual Rainfall}$ for a specific Control Depth as determined by the previous equation

$\% \text{ Annual Rainfall }_{AR}$ = This will always be 88%

The same basic approach was used to define maximum mass nutrient reduction rates for storms above the anchor rate, up to the 2.5 inch storm event. In general, no BMP performance monitoring data is available in the literature to evaluate removal for runoff treatment depths beyond 1.5 inches, so this conservative approach was used for the extrapolation. The Panel had limited confidence in removal rates in the 1.5 to 2.5 inch range, although it was not overly concerned with this limitation, since few of any retrofits are sized to capture that much runoff. A spreadsheet that defines how the anchor rates and bypass adjustments were derived can be obtained from the Chesapeake Stormwater Network (CSN).

The tabular data was converted into a series of curves to make it easier for users to define a rate for the unique combination of runoff capture volume and degree of runoff reduction. This was done by fitting a log-normal curve to the tabular data points, which came within a few percentage points of the tabular values for a wide range of runoff capture depths and removal rates.

A 0.05 inch runoff capture volume was established as the cut-off point for getting any retrofit removal rate, since this roughly corresponds to the depth of initial abstraction that occurs on impervious surface. It should be noted that retrofits in this small size range will require very frequent maintenance to maintain their performance over time.

The Panel concluded that the generalized retrofit removal adjutor curves were a suitable tool for estimating the aggregate pollutant load reductions associated with hundreds or even thousands of future retrofit projects at the scale of the Bay watershed and the context of the Chesapeake Bay Watershed Model.

7-C.2.1 Notes on the Standard Retrofit Equation

The specific retrofit storage volume achieved at an individual site is usually "discovered" and is measured or estimated by an engineer based on site constraints. The retrofit storage volume (usually reported in acre-feet) needs to be converted into the appropriate unit on the X-axis of the curves (i.e., depth of runoff captured by retrofit per impervious acre).

The basic rationale is that the Rainfall Frequency Analysis method used to derive the adjutor curve (above and below the anchor points) is based on the assumption that the runoff delivered to a practice is generated from a unit impervious acre. By contrast, the retrofit storage volume available at each retrofit is unique, based on the upstream land cover, soils and the drainage area. Consequently, the retrofit storage volume must be adjusted to get a standard depth of runoff treatment per unit impervious cover to get the correct depth to use on the x-axis of the retrofit adjutor curves.

This is done by using standard retrofit equation which multiplies the retrofit storage volume by 12 to get acre-inches, and then is divided by the impervious acres to get the desired unit for the retrofit adjutor curves. Numerically, the standard retrofit equation is:

$$= \frac{(RS)(12)}{IA}$$

The removal rates determined from the retrofit removal adjutor curves are applied to the entire drainage area of the retrofit, and not just its impervious acres. Also, the retrofit reporting unit is the entire treated area, regardless of whether it is pervious or impervious.

7-C.2.2 Notes on the Derivation of Sediment Removal Rates

The original retrofit removal rate adjutor table (CSN, 2011) did not include estimates for sediment removal. They were derived in January of 2012 after a detailed analysis of BMP sediment removal rates drawn from the following sources – Brown and Schueler, (1997), Winer (2000), Baldwin et al, (2003), CWP (2007), Simpson and Weammert, (2009), and ISBD (2011a). Collectively, these BMP performance research reviews analyzed more than 200 individual urban BMP performance studies conducted both within and outside of the Chesapeake Bay watershed. The following general conclusions were drawn from the analysis.

Sediment removal by both traditional BMPs and LID practices was consistently higher and less variable than nutrient removal. This is attributed to the particulate nature of sediment which makes it easier to achieve reductions through settling, trapping, filtering and other physical mechanisms.

The analysis began with an examination of existing CBP-approved rates (see **Table 7-C.3**). Two important trends were noted. First, TSS removal always exceeded TP and TN rates for every category of urban BMP. Second, nearly all the rates were within a fairly narrow range of 60 to 90%.

Table 7-C.3. Approved CBP BMP Efficiency Rates for Retrofit Analysis ^{1, 2, 3}

| URBAN BMP | | TOTAL NITROGEN | TOTAL PHOSPHORUS | TSS |
|---|--------------|-------------------------|------------------|-----|
| | | MASS LOAD REDUCTION (%) | | |
| Wet Ponds and Constructed Wetlands | | 20 | 45 | 60 |
| Dry Detention Ponds | | 5 | 10 | 10 |
| Dry Extended Detention Ponds | | 20 | 20 | 60 |
| Infiltration | | 80 (85) ⁴ | 85 | 95 |
| Filtering Practices (Sand Filters) | | 40 | 60 | 80 |
| Bioretention | C & D w/UD | 25 | 45 | 55 |
| | A & B w/ UD | 70 | 75 | 80 |
| | A & B w/o UD | 80 | 85 | 90 |
| Permeable Pavement | C & D w/UD | 10 (20) | 20 | 55 |
| | A & B w/ UD | 45 (50) | 50 | 70 |
| | A & B w/o UD | 75 (80) | 80 | 85 |
| Grass Channels | C & D w/o UD | 10 | 10 | 50 |
| | A & B w/o UD | 45 | 45 | 70 |
| Bioswale aka dry swale | | 70 | 75 | 80 |
| ¹ In many cases, removal rates have been discounted from published rates to account for poor design, maintenance and age, and apply to generally practices built prior to 2008 ² Current Practices are designed to more stringent design and volumetric criteria, and may achieve higher rates –see Table A-4 ³ Some practices, such as forest conservation, impervious cover reduction, tree planting are modeled as a land use change. Urban stream restoration is modeled based on a reduction per linear foot of qualifying stream restoration project ⁴ Numbers in parentheses reflect design variation with a stone sump to improve long term infiltration rates | | | | |

The same composite BMP method was employed using the CBP-approved rates to define sediment removal rates for RR and ST practices. The ST practice category included wet ponds, constructed wetlands and sand filters, which collectively had a TSS removal rate of 70%. The RR category included all design variations of bioretention, permeable pavement, infiltration and bio-swales in **Table 7-C.3**, and had a slightly higher composite TSS removal rate of 75%.

Other BMP performance reviews have also noted that TSS removal rates exceed TP or TN removal rates for all individual studies of traditional urban BMPs (up to 1.0 inch of runoff treated, Winer, 2000 and CWP, 2007).

The sediment removal rate for traditional BMPs is ultimately limited by particle size considerations. Studies have shown that there is an irreducible concentration associated with the outflow from traditional BMPs (Winer, 2000 and NRC, 2008) around 15 to 20 mg/l which reflects the limits of settling for the most fine-grained particles. In practical terms, this sets an upper limit on maximum sediment removal around 70 to 80% for the range of monitored BMPs (i.e., sized to capture 0.5 to 1.5 inches of runoff).

Additional analysis was done to examine whether sediment removal rates for LID practices (i.e., runoff reduction practices) would achieve high rates of runoff reduction. Recent sediment mass removal rates were reviewed for bioretention, permeable pavers, green roofs, rain tanks, rooftop disconnection and bioswales (Simpson and Weammert, 2009, ISBD, 2011a, and a re-analysis of individual studies contained in CWP and CSN, 2008). The following general conclusions about LID sediment removal rates were drawn from the analysis:

- Most LID practices had lower TSS loadings than traditional BMPs, primarily because there was no major up-gradient sediment source area (e.g., green roofs, rain tanks, permeable pavers, rooftop disconnection) or a small contributing drainage area (bioretention, bio-swales).
- In general, LID practices had a slightly lower outflow sediment concentration than their traditional BMP counterparts (around 10 mg/l-- ISBD, 2011a).
- The ability of LID practices to change the event mean concentration of sediment as it passed through a practice differed among the major classes of LID practices. For example, nearly a dozen studies showed that bioretention and bioswales could achieve significant reduction in sediment concentrations. On the other hand, permeable pavers and green roofs generally produced low or negative changes in sediment concentrations through the practice. This finding was not deemed to be that important given how low the sediment inflow concentrations were.

Based on these conclusions, the Panel took a conservative approach and did not assign higher sediment removal rates for LID practices that achieved a high rate of runoff reduction, at least for facilities designed to capture less than an inch or more of runoff.

Beyond that point, the Panel did assign a modest increase in sediment removal rate for LID practices under the assumption that the combination of high runoff capture and reduction would work to reduce or prevent accelerated downstream channel erosion. The Panel notes that the extra sediment removal rate for this range of LID practices is an untested hypothesis that merits further research.

7-C.2.3 Notes on Revising the TN Adjustor Curve to Reflect Base flow Nitrate Movement in Urban Watersheds

The adjustor curves are used to define a removal rate that applies to both the pervious and impervious areas in the contributing drainage areas for the stormwater treatment practices. The removal rates properly apply to surface runoff and some portion of the interflow delivered to the

stream, but may not properly apply to groundwater export of nitrate-nitrogen from the urban landscape. The "missing" nitrate may be nitrate that exits a runoff reduction via infiltration into soil, or slow release through an under drain (e.g., bioretention).

Once stormwater runoff is diverted to groundwater, the overall load is reduced by using the ground as a filtering medium, but not eliminated. Therefore, the WTWG concluded that the original TN adjustor curves developed by the expert panel may over-estimate TN removal rates, and should be discounted to reflect the movement of untreated nitrate from runoff reduction BMPs. This discounting is not needed for TKN, TP or TSS as these pollutants are not mobile in urban groundwater.

The USWG concurred with this approach and developed the following procedure to derive a new TN adjustor curve to account for groundwater nitrate migration from runoff reduction practices. The basic approach is documented in Schueler (2012a and 2012b).

This discount factor is fairly straight forward to calculate and is simply based on the ratio of nitrate in relation to total nitrogen found in urban stormwater runoff. Stormwater runoff event mean concentration data from the National Stormwater Quality Database (Pitt et al, 2004) was analyzed for more than 3000 storm events, and the nitrate-to-TN fraction was consistently around 0.3. This sets an upper boundary on the fraction of the inflow nitrate concentration to the BMP which could be lost to groundwater or under drains at about 30%.

The next step is to account for any nitrate loss within the BMP due the combination of either plant uptake and storage and/or any de-nitrification within the BMP. Most runoff reduction practices employ vegetation to promote ET and nutrient uptake, whereas the de-nitrification process is variable in both space and time.

Over 70 performance studies have measured nitrate removal within runoff reduction BMPs. A summary of the national research is shown in **Table 7-C.4** below. Clearly, there is a great deal of variability in nitrate reductions ranging from nearly 100% to negative 100% (the negative removal occurs when organic forms of nitrogen are mineralized/nitrified into nitrate within the BMP).

Some well studied runoff reduction practices, such as bioretention and bioswales, have a median nitrate removal ranging from 25 to 45%, presumably due to plant uptake. Initial results for green roofs indicate moderate nitrate reduction as well. Non-vegetative practices, such as permeable pavers and a few infiltration practices, show zero or even negative nitrate removal capability (see **Table 7-C.4**). Submerged gravel wetlands that create an aerobic/anaerobic boundary that promotes denitrification appear capable of almost complete nitrate reduction.

Therefore, it is recommended that maximum nitrate removal within runoff BMPs be assumed to be no more than 40%. Although this value may seem generous, it should be noted that some additional nitrate reduction occurs as the nitrate moves down-gradient through soils on the way to the stream. Under this conservative approach, no additional nitrate reduction is assumed after it exits the BMP and migrates into groundwater.

Given the nitrate inflow concentrations, the potential groundwater/under drain nitrate loss would be $(0.3)(0.60) = 0.18$, or a discount factor of 0.82

The discount factor is then applied to the anchor rates used to derive a new TN adjustor curve. The anchor rate for RR practices would be adjusted downward from the current 70% to 57%, and the existing runoff frequency spectrum equation would be used to develop a new, lower curve for TN removal. An example of the how this discount influences the existing TN adjustor curve is shown in **Figure 7-C.1** below.

It is also noted that no nitrate loss parameter needs to be defined for stormwater treatment (ST) practices, since inlet and outlet monitoring of these larger facilities already takes this into account (and is a major reason why the ST curve is so much lower than the RR curve).

The de-nitrification process can be enhanced through certain design features (inverted under drain elbows, IWS, enhanced media). Several good research reviews indicate that these design features show promise in enhancing nitrate removal (Kim et al, 2003, NCSU, 2009, Weiss et al, 2010), these features are not currently required in Bay state stormwater manuals. Should future research confirm that these features can reliably increase nitrate removal through denitrification and/or plant uptake, it is recommended that a future expert panel revisit the existing nitrogen adjustor curve.

Table 7-C.4 Nitrate Removal by Runoff Reduction Practices ¹

| Practice | Median Removal Rate | No. of Sites | Range | Source |
|---------------------------|---------------------|--------------|-------------|-----------------|
| Bioretention ² | 43% | 9 | 0 to 75 | CWP, 2007 |
| Bioretention ² | 44% | 1 | NA | UNH, 2009 |
| Bioretention ² | 24% | 10 | NA | ISBD, 2010 |
| Bioswales | 39% | 14 | -25 to 98 | CWP, 2007 |
| Bioswales | 7% | 18 | NA | ISBD, 2010 |
| Infiltration ³ | 0 | 5 | -100 to 100 | CWP, 2007 |
| Permeable Pavers | -50% ⁴ | 6 | NA | IBSD, 2010 |
| Permeable Pavers | 0 | 4 | | Collins, 2007 |
| Green Roof ⁵ | Positive | 4 | NA | Long et al 2006 |
| Gravel Wetland | 98% | 1 | NA | UNH, 2009 |

Notes:

¹ As measured by change of event mean concentration (EMC) entering device and final exfiltrated EMC, and involves either or plant uptake or denitrification

² For "conventional" runoff reduction practices only, i.e., no specific design features or media enhancements to boost nitrate removal

³ Category includes several permeable paver sites

⁴ A negative removal rate occurs when organic forms of nitrogen are nitrified to produce additional nitrate which is

⁵ Test column study

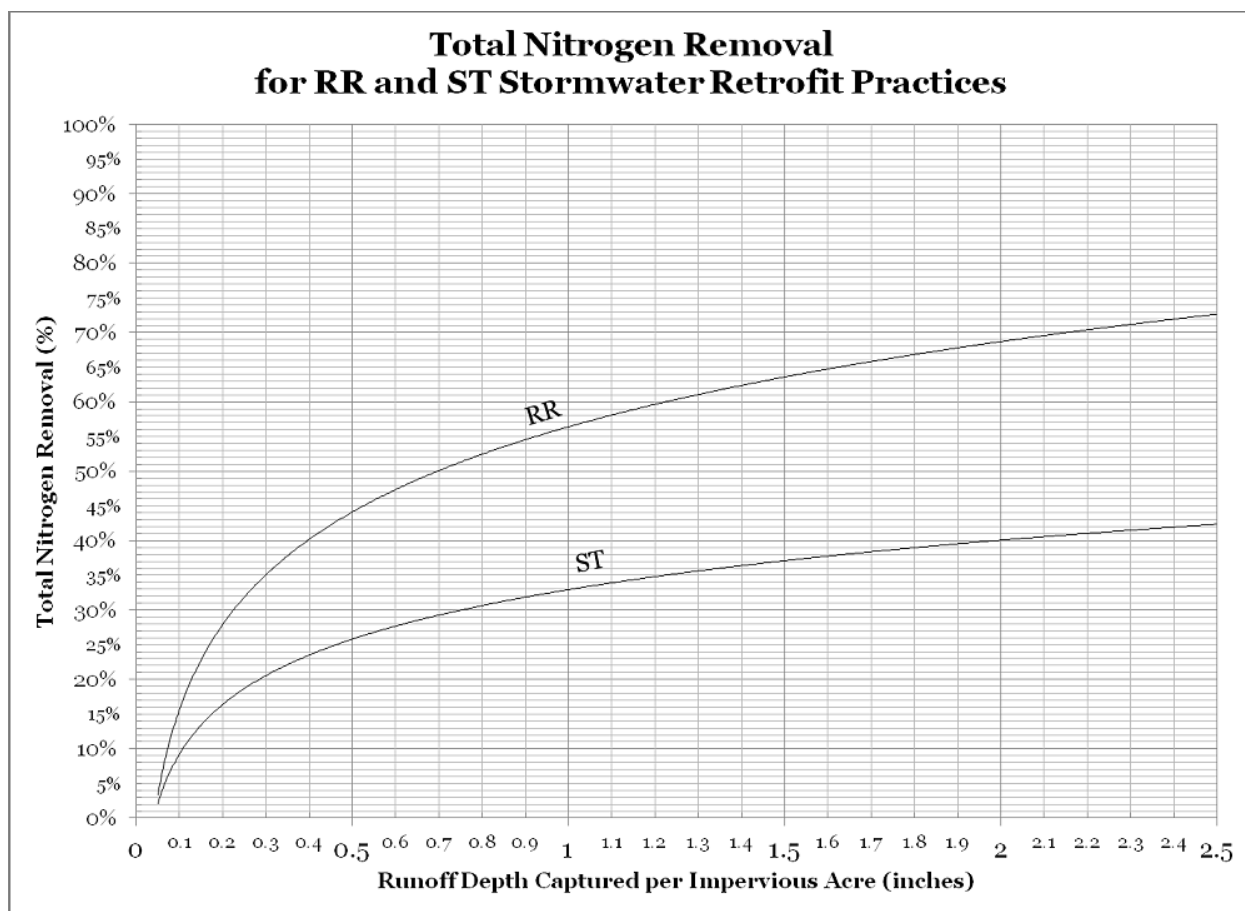


Figure 7-C.1. Revised TN Adjustor Curve

Text would be added to memos that acknowledge the “escaped nitrate” issue up gradient and down gradient of the BMP that might not be effectively captured by the BMP, but indicate that this should be resolved in the next version of CBWM.

7-C.3.0 REFERENCES

Baldwin, A., T. Simpson and S. Weammert. 2003. Reports of urban BMP efficiencies. Prepared for EPA Chesapeake Bay Program. Urban Stormwater Workgroup. University of Maryland, College Park

Brown, W. and T. Schueler. 1997. National Pollutant Removal Database for Stormwater BMPs. First Edition. Center for Watershed Protection. Ellicott City, MD.

CWP. 2007. *National Pollutant Removal Performance Database Version 3.0*. Center for Watershed Protection, Ellicott City, MD.

CWP and Chesapeake Stormwater Network (CSN). 2008. *Technical Support for the Baywide Runoff Reduction Method*. Baltimore, MD www.chesapeakestormwater.net

ISBD. 2011a. International stormwater best management practice database pollutant category summary: solids (TSS, Turbidity and TDS). Prepared by Geosyntec Consultants and Wright Water Engineers.

Kim, H., E. Seagren, and A. Davis. 2003. Engineering bioretention for removal of nitrate in stormwater. *Water Environment Research* 75(4);355-367

National Research Council (NRC). 2008. *Stormwater Management in the United States*. National Academy of Science Press www.nap.edu Washington, DC.

North Carolina State University. 2009. Designing bioretention with an internal water storage layer. *Urban Waterways*.

Pitt, R., T. Brown and R. Morchque. 2004. *National Stormwater Quality Database. Version 2.0*. University of Alabama and Center for Watershed Protection. Final Report to U.S. Environmental Protection Agency.

Schueler, T. 2012a. June 6, 2012 Memo to Expert Panels. Watershed Technical Workgroup Responses to Final Recommendation Report. Chesapeake Stormwater Network, Baltimore, MD.

Schueler, T. 2012b. July 2, 2012 Memo to Urban Stormwater Group and Expert Panels. Resolution of Technical Issues Related to the Urban Retrofit and Performance Standards Expert Panel Recommendation. Chesapeake Stormwater Network, Baltimore, MD.

Simpson, T. and S. Weammert. 2009. Developing nitrogen, phosphorus, and sediment efficiencies for tributary strategy practices. BMP Assessment Final Report. University of Maryland Mid-Atlantic Water Program. College Park, MD.

Weiss, P., J. Gulliver, A. Erickson, 2010. The performance of grass swales as infiltration and pollution prevention practices. A Literature Review. University of Minnesota. Stormwater Center.

Winer, R. 2000. National pollutant removal database for stormwater treatment practices. 2nd edition. EPA Office of Science and Technology. Center for Watershed Protection. Ellicott City, MD